

TRAFFIC IMPACT STUDY AND TRAFFIC SIGNAL WARRANT ANALYSIS

TIPTON, INDIANA



Prepared for:

City of Tipton

Submitted by:



February 2023

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1 INTRODUCTION

Jefferson St. (formerly State Road 28) and Main St. (formerly State Road 19) are the two main corridors in Tipton, Indiana. Recently, Indiana Department of Transportation (INDOT) relinquished control of SR 28 throughout Tipton, as well as Main St. from Park Ave. to Jefferson St. State Road 28 is now discontinuous, stopping at County Road 560 W and starting again at the Ash Rd/State Road 19 intersection on the east end of Tipton. State Road 19 now turns east on Park Rd and connects back to existing SR 19 at the Ash Rd. intersection with Jefferson St. The full relinquishment area is shown as a map in Figure 1.



Figure 1 – Map of Tipton, IN showing relinquished segments

There are four (4) INDOT traffic signals within the relinquished segments that are now controlled by Tipton, as shown in Figure 2.

1. Jefferson St. at West St.
2. Jefferson St. at Main St.
3. Jefferson St. at Independence St.
4. Main St. at Madison St.



Figure 2 – Location of Tipton's newly controlled traffic signals

Breaking up SR 28 and rerouting SR 19, as well as restricting heavy truck traffic along the old routes, is thought to have impacted traffic volumes on both Main St. and Jefferson St. Due to this apparent decrease in traffic, Tipton contracted with WSP USA Inc. to perform a traffic impact study and signal warrant analysis to determine potential future intersection control options for the intersections. This study analyzed the existing conditions of the signalized grid, determined if the traffic signals are still warranted based on Indiana guidelines, and identified future operations for traffic control alternatives at each intersection if signals are no longer warranted.

2 EXISTING CONDITIONS

Downtown Tipton consists of closely spaced city streets and includes four signals in a T pattern, with signals on Jefferson St. for three consecutive intersections. The middle intersection is with Main St., which has a signal a block south at Madison St. INDOT managed the signal infrastructure and timings at these four intersections prior to relinquishment. While INDOT investigates unsignalized intersections using the Indiana Manual on Uniform Traffic Control Devices (IMUTC) traffic signal warrant guidance to determine if the intersection should be signalized, the analysis to determine that existing signals still meet the guidelines is rarely investigated. In order to help understand if these intersections could be converted to stop-controlled, the existing signal operations were analyzed.

2.1 EXISTING TRAFFIC VOLUMES

Traffic counts were conducted from 6:00 AM – 7:00 PM at each intersection within the study area. These were reviewed to identify the morning (AM) and evening (PM) peak hours of travel. The AM peak hour is from 7:15-8:15AM, and the PM peak hour is from 4:30-5:30 PM. These peak hour volumes were then utilized to examine the existing signal operations, as well as the possible alternatives discussed later in the report. Figure 3 summarizes the existing AM and PM Peak hour turning movements at all four intersections. The detailed traffic counts at each intersection can be found in Appendix A

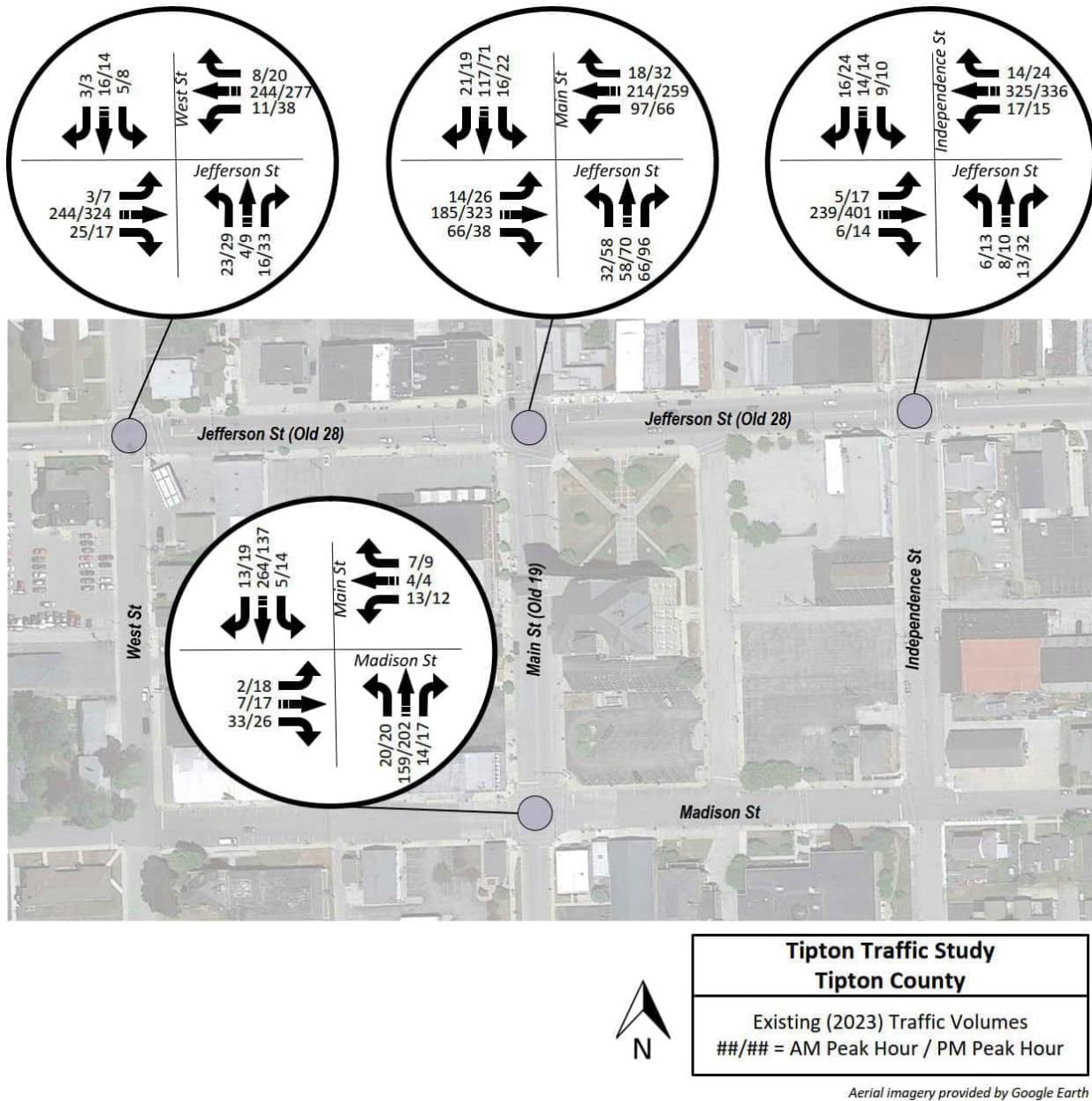


Figure 3 – Existing (2023) AM and PM Peak Hourly Volumes

2.2 EXISTING OPERATIONAL ANALYSIS

Existing volumes were input into a Synchro model to determine how well the signal system currently functions. Signal timings in the Synchro model were optimized based on the peak hour volumes and spacing between intersections to provide the smallest delay possible. The evaluation began by looking at the Level of Service (LOS) of all movements at each intersection. These LOS ratings are measured in terms of average delay, where delay is a measure of driver discomfort, frustration, fuel consumption, and lost travel time. LOS A is the best operating condition, and LOS F has the longest delays, therefore making it the worst operating condition. **LOS D or better is considered acceptable in urban settings**, although LOS C or better is desirable. The LOS criteria for intersections is provided in the 2010 Highway Capacity Manual (HCM) and summarized in Table 1 below.

Level of Service	Description	Average Control Delay Per Vehicle (seconds)	
		Unsignalized Intersection	Signalized Intersection
A	Little or no delay.	≤ 10.0	≤ 10.0
B	Short traffic delays.	$> 10.0 \text{ and } \leq 15.0$	$> 10.0 \text{ and } \leq 20.0$
C	Average traffic delays.	$> 15.0 \text{ and } \leq 25.0$	$> 20.0 \text{ and } \leq 35.0$
D	Long traffic delays.	$> 25.0 \text{ and } \leq 35.0$	$> 35.0 \text{ and } \leq 55.0$
E	Very long traffic delays.	$> 35.0 \text{ and } \leq 50.0$	$> 55.0 \text{ and } \leq 80.0$
F	Demand exceeds capacity resulting in extreme delays and queuing.	> 50.0	> 80.0

Table 1 – Level of Service (LOS) Criteria for Intersections

The results from the Synchro analysis of existing conditions are shown in Table 2. Although the LOS remains acceptable for all movements, delays on the minor approaches should be noted. Traffic signals that are closely spaced operate best when in coordination with each other. When signals are coordinated, the minor approach movements can't be served immediately but only when their scheduled time arrives so that the mainline greens can remain coordinated. This can lead to vehicles on the side street approaches sitting at a red light even when there is little or no traffic on the mainline, leading to driver frustration.

Intersection	Approach	Movement	LOS		Delay (s/veh)		95 th Percentile Queue (ft)	
			AM	PM	AM	PM	AM	PM
Jefferson St. & West St.	Eastbound	LT	A	A	3.7	3.6	2	4
		TH/RT	A	A	3.6	4.2	60	80
	Westbound	LT	A	A	2.5	2.7	4	9
		TH/RT	A	A	2.3	2.7	38	45
	Northbound	LT/TH/RT	C	B	20.6	18.7	31	41
Jefferson St. & Main St.	Southbound	LT/TH/RT	C	C	23.7	22.9	20	23
		LT	A	A	3.6	3.4	4	7
		TH/RT	A	A	4.4	4.4	28	47
	Westbound	LT	A	A	4.2	3.4	15	12
		TH/RT	A	A	4.2	4.0	28	34
	Northbound	LT	B	C	17.8	21.2	14	47
		TH/RT	B	B	10.8	11.7	10	64
Jefferson St. & Independence St.	Southbound	LT	B	C	19.2	21.5	17	21
		TH/RT	C	C	24.0	20.5	75	51
	Eastbound	LT	A	A	2.6	2.7	2	5
		TH/RT	A	A	2.3	2.8	36	66
	Westbound	LT	A	A	3.6	3.7	7	7
		TH/RT	A	A	3.7	3.8	80	87
Main St. & Madison St.	Northbound	LT/TH/RT	B	B	18.2	16.8	19	36
	Southbound	LT/TH/RT	B	B	19.6	18.1	29	31
	Northbound	LT/TH/RT	A	A	3.6	4.3	40	61
	Southbound	LT/TH/RT	A	A	3.8	3.1	73	38
Eastbound	LT/TH/RT	B	B	13.8	19.8	15	36	
	Westbound	LT/TH/RT	C	B	21.3	19.0	22	19

Table 2 – Summary of Existing Signal Operation

2.3 TRAFFIC SIGNAL WARRANT ANALYSIS

Traffic signals are a useful tool in traffic operations, however they are not a one-size-fits-all solution. A traffic signal assigns the right-of-way to the various traffic movements, helping minor movements proceed when the major movements are busy, and also helping progress vehicles efficiently through multiple intersections. Signal warrants are performed to determine whether installation of a traffic signal is justified at a particular location. There are nine warrants, with the criteria for each provided in the Indiana Manual on Uniform Traffic Control Devices (IMUTCD). The most commonly used signal warrants are the eight-hour vehicular volume warrant and the peak hour warrant.

The eight-hour warrant includes two conditions. Each condition establishes a minimum volume of total traffic on the mainline and a minimum of total traffic on the side street approach with the heaviest volume, based on the conditions of the intersection. Both approaches must meet the minimum volume of either Condition A or Condition B for at least eight hours of a 24-hour day to meet this traffic signal warrant. Condition A was established for locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal. Condition B was established for locations where Condition A is not satisfied and where the traffic volume on a major street is so heavy that the side street suffers undue delay.

A peak hour warrant is intended for use at locations where traffic conditions result in undue delay on side street approaches for one hour of a 24-hour period. This warrant is typically used at intersection near facilities that attract or discharge large numbers of vehicles, such as shift changes at a large factory.

A traffic signal warrant analysis was completed at all four intersections. The eight-hour warrant and peak hour warrants are discussed below, with results from each fully shown in Appendix C.

2.3.1 EIGHT-HOUR TRAFFIC SIGNAL WARRANT ANALYSIS

Counts were collected at each intersection. These volumes were compared against signal warrant standard volumes to determine if either warrant was met. Unlike other hourly warrants, the eight-hour warrant doesn't require the criteria to be met for eight consecutive hours, but rather for any eight hours within a 24-hour day.

The IMUTCD eight-hour signal warrant traffic volume thresholds depend on the number of lanes for each approach, as well as speed limit of the road and size of the community. These thresholds are provided in Table 3. Tipton has a population of 5,275 as of the 2020 census, meaning the study may use the 70% volume requirements noted below in note c. To ensure the analysis considered a worst-case scenario, the thresholds for 8-hour warrant used the 70% values shown in Table 3.

Condition A—Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volum minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B—Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volum minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

^a Basic minimum hourly volume

^b Used for combination of Conditions A and B after adequate trial of other remedial measures

^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

^d May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

Table 3 – Eight-Hour Signal Warrant Vehicle Volume Thresholds

At the intersection of Jefferson St. & West St. the 8-hour warrant was not met in either Condition A or Condition B. Condition A met 0 hours of the day and Condition B met 3 hours. Full results are shown in Appendix C.

At the intersection of Jefferson St. & Main St. the 8-hour warrant was not met in either Condition A or Condition B. Condition A met 5 hours of the day and Condition B met 3 hours. Full results are shown in Appendix C.

At the intersection of Jefferson St. & Independence St. the 8-hour warrant was not met in either Condition A or Condition B. Condition A met 0 hours of the day and Condition B met 2 hours. Full results are shown in Appendix C.

At the intersection of Main St. & Madison St. the 8-hour warrant was not met in either Condition A or Condition B. Condition A met 0 hours of the day and Condition B met 0 hours. Full results are shown in Appendix C.

2.3.2 PEAK HOUR TRAFFIC SIGNAL WARRANT ANALYSIS

A peak hour warrant is traditionally used for unusual cases, such as office complexes or manufacturing plants, that attract or discharge large numbers of vehicles over a short time. While none of the four signals downtown Tipton seem to fit this theme, the FCA Transmission Plant west of town is a large volume generator. Although that plant likely doesn't seriously affect downtown intersections, because the peak hour warrant is the second most common warrant used in signal analysis it's worth investigating whether or not it is met.

The category of isolated town with a population under 10,000 again applies, meaning only 70% of the threshold volumes are required to meet the warrant. The peak hour warrant thresholds depend on the number of lanes for each approach and are a relationship between total vehicles on both directions of the major road and vehicles on the busier minor approach. These changing thresholds can be seen by the black lines Figure 4.

The peak hour warrant was not met for any of the four intersections. Jefferson St. & Main St. was the closest during the PM Peak with 744 vehicles on both Jefferson St. approaches and 224 vehicles on the northbound Main St. approach, but those volumes fall below the line for 2 or more lanes on all approaches, as shown by the red diamond in Figure 4.

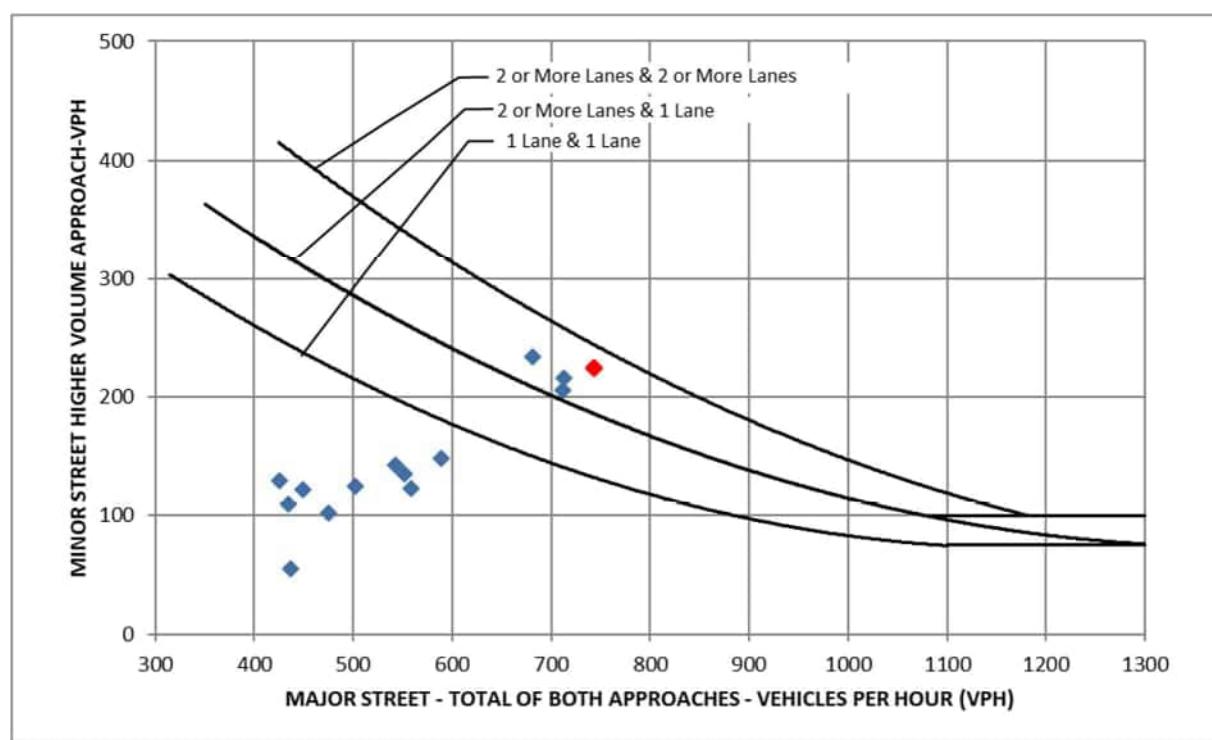


Figure 4 – Peak Hour Warrant Volume Diagram at Jefferson St. & Main St. (70% Factor)

3 CONVERTING INTERSECTION TO STOP-CONTROLLED

The signal warrant analysis stated in the previous section determined that the four intersections analyzed do not currently warrant a traffic signal. The next step was to determine which intersection control method would be best if signals are removed. The only methods that were analyzed were two-way stop-controlled and all-way stop-controlled intersections; while roundabouts are a great alternative to traffic signals in terms of safety and in some cases delay, this alternative wasn't considered due to cost and footprint needs.

3.1 SCENARIO ONE: TWO-WAY STOP-CONTROLLED

The first alternative investigated would be to convert the intersections to two-way stop-controlled (TWSC), where the side street would have a stop sign and the mainline would operate as free flow. For the intersections within the study area, Jefferson St. would be the mainline at all three of its intersections, and Main St. would be at Madison St. This scenario would retain the preferential treatment of Jefferson St. that the signalized operation provides.

3.1.1 TWO-WAY STOP-CONTROLLED OPERATION

Table 4 shows the LOS, delay, and queuing at each intersection when the Synchro model analyzed all intersections as TWSC. Jefferson St. movements are anticipated to operate at LOS A, which would be expected given free flow operation. The expected operation on the side street approaches were all LOS C or better at all intersections except Jefferson St. at Main St. At this intersection, the analysis indicated unacceptable operations on the side street approaches if the mainline approaches weren't controlled. The AM peak displayed LOS F for the northbound left turn lane and LOS E for the southbound through/right turn lane. Similarly, the PM peak displayed LOS E for both northbound and southbound left turn lanes. Not only are these results considered unacceptable under Highway Capacity Manual guidelines, but they are also worse than the current signalized operation.

Intersection	Approach	Movement	LOS		Delay (s/veh)		95 th Percentile Queue (ft)	
			AM	PM	AM	PM	AM	PM
Jefferson St. & West St.	Eastbound	LT	A	A	7.9	8	0	0
		TH/RT	A	A	0	0	0	0
	Westbound	LT	A	A	8.0	8.1	1	3
		TH/RT	A	A	0	0	0	0
	Northbound	LT/TH/RT	C	C	15.4	16.4	15	20
Jefferson St. & Main St.	Southbound	LT/TH/RT	C	C	15.7	17.9	11	10
		LT	A	A	7.8	8.0	1	2
		TH/RT	A	A	0	0	0	0
	Westbound	LT	A	A	8.2	8.2	8	5
		TH/RT	A	A	0	0	0	0
	Northbound	LT	F	E	61.5	46.9	52	54
		TH/RT	C	C	21.8	22.4	68	67
	Southbound	LT	D	E	34.8	42.0	14	23
		TH/RT	E	D	38.1	25.9	109	53
Jefferson St. & Independence St.	Eastbound	LT	A	A	8	8.1	0	1
		TH/RT	A	A	0	0	0	0
	Westbound	LT	A	A	7.9	8.4	1	1
		TH/RT	A	A	0	0	0	0
	Northbound	LT/TH/RT	B	C	13.8	17	8	15
Main St. & Madison St.	Southbound	LT/TH/RT	B	C	14.6	17.4	10	16
	Northbound	LT/TH/RT	A	A	1.1	0.8	2	1
	Southbound	LT/TH/RT	A	A	0.2	0.7	0	1
	Eastbound	LT/TH/RT	B	B	13.0	12.4	12	13
	Westbound	LT/TH/RT	C	B	17.7	12.8	8	6

Table 4 – Summary of Operations: All Intersections Two-Way Stop-Controlled

3.2 SCENARIO TWO: ALL-WAY STOP-CONTROLLED

A second alternative to signalized intersections would be converting the four intersections to all-way stop-controlled (AWSC). This alternative would slow down progression on Jefferson St., but would decrease delays for side street movements and limit the ability for drivers on Jefferson St. to speed through town. However, a drawback of AWSC is a tendency for drivers to ignore the stop signs when they rarely have to stop for conflicting traffic.

The results from changing every intersection to AWSC are shown in Table 5. No intersections are anticipated to experience operations worse than LOS C. In this scenario there are fewer movements experiencing LOS A than in the other alternatives, which is also to be expected due to all vehicles stopping in AWSC. There is also more queuing in AWSC than the other alternatives because Jefferson St. has many more vehicles than the minor movements and forcing them to stop will cause larger queues during the peak hours.

Intersection	Approach	Movement	LOS		Delay (s/veh)		95 th Percentile Queue (ft)	
			AM	PM	AM	PM	AM	PM
Jefferson St. & West St.	Eastbound	LT	A	A	8.4	8.6	1	1
		TH/RT	B	B	12.9	13.4	58	55
	Westbound	LT	A	A	8.5	8.9	2	4
		TH/RT	B	B	11.7	12.6	44	51
	Northbound	LT/TH/RT	A	A	9.3	9.5	6	9
Jefferson St. & Main St.	Southbound	LT/TH/RT	A	A	9.2	9.3	4	5
		LT	B	B	10.5	10.5	2	4
	Eastbound	TH/RT	C	C	19.9	24.2	82	112
		LT	B	B	12.5	11.4	18	10
	Westbound	TH/RT	B	C	16.8	19.8	58	81
		LT	B	B	11.7	12.2	8	13
	Northbound	TH/RT	B	B	14.5	14.3	40	37
Jefferson St. & Independence St.	Southbound	LT	A	B	11.2	11.7	4	4
		TH/RT	B	B	15	13.2	38	22
	Eastbound	LT	A	A	8.4	8.8	1	2
		TH/RT	B	C	11.6	19.2	44	109
	Westbound	LT	A	A	8.5	8.8	2	3
		TH/RT	B	C	12.8	15.3	58	78
Main St. & Madison St.	Northbound	LT/TH/RT	A	A	8.9	9.7	4	6
	Southbound	LT/TH/RT	A	A	9.0	9.8	6	9
	Northbound	LT/TH/RT	B	A	11.3	9.9	44	32
	Southbound	LT/TH/RT	B	A	12.9	8.9	65	19
	Eastbound	LT/TH/RT	A	A	8.9	8.5	5	8
	Westbound	LT/TH/RT	A	A	9.1	8.3	7	4

Table 5 – Summary of Operations: All Intersections All-Way Stop-Controlled

3.3 SCENARIO THREE: MIX OF STOP-CONTROL

A third alternative would be to mix scenarios one and two, with TWSC for Jefferson St. at West St., Jefferson St. at Independence St., and Main St. at Madison St., and AWSC for Jefferson St. at Main St.

Since these alternatives were previously analyzed, the results are the same as in Table 4 (TWSC) and Table 5 (AWSC) for each respective intersection type. The next section will compare expected operations for all three proposed scenarios with existing operations.

3.4 COMPARING ALL SCENARIOS WITH EXISTING CONDITIONS

In order to compare the scenarios against each other, Table 6 examines the average delay across the entire intersection. The total intersection delay is the average delay that each vehicle experiences. These results, combined with the earlier LOS results for each scenario, assist in determining a preferred scenario and recommendation.

A new delay value was calculated and shown below in Table 6 to give an overall comparison of all three scenarios: total delay. This value took the intersection delay in seconds per vehicle one step further by normalizing the delay of all vehicles using all four intersections. This helped balance the results because the total volume at Jefferson St. at Main St. is about 50% more than the average total volume at the other three. Notice here that the TWSC scenario has the smallest total delay during both peak hours, however this method produced unacceptable LOS for Main St. movements at Jefferson St. as previously discussed. The AWSC scenario shows total delay more than twice as much as the current signal operation. The total delay in Scenario 3 is the closest acceptable alternative to the current signal operation.

Scenario	Intersection	Intersection LOS		Intersection Delay (s/veh)		Total Delay Peak Hours (s/veh)
		AM Peak	PM Peak	AM Peak	PM Peak	
Existing Signal Operation	Jefferson St. & West St.	A	A	5.7	5.8	6.7
	Jefferson St. & Main St.	A	A	9.5	8.5	
	Jefferson St. & Independence St.	A	A	5.0	5.0	
	Main St. & Madison St.	A	A	5.2	7.1	
Scenario 1: Two-Way Stop-Controlled	Jefferson St. & West St.	A	A	2.4	2.9	5.7
	Jefferson St. & Main St.	B	B	14.3	10.8	
	Jefferson St. & Independence St.	A	A	2.0	2.3	
	Main St. & Madison St.	A	A	2.1	3.0	
Scenario 2: All-Way Stop-Controlled	Jefferson St. & West St.	B	B	11.9	12.3	14.1
	Jefferson St. & Main St.	C	C	16.1	18.2	
	Jefferson St. & Independence St.	B	C	11.7	16.2	
	Main St. & Madison St.	B	A	11.8	9.3	
Scenario 3: Mix of TWSC and AWSC	Jefferson St. & West St.	A	A	2.4	2.9	7.4
	Jefferson St. & Main St.	C	C	16.1	18.2	
	Jefferson St. & Independence St.	A	A	2.0	2.3	
	Main St. & Madison St.	A	A	2.1	3.0	

Table 6 – Intersection LOS & Delay for Each Scenario

3.5 CONCLUSIONS

The results from Section 2 showed that although the existing conditions are operating well overall, the traffic signals aren't warranted and even with optimal signal timings add unnecessary delay to the minor movements. Removing the signals and changing to stop-control for the intersection was determined to be best for traffic operations in the

downtown grid. It should be noted that IMUTCD Section 4B.02 discusses the appropriate steps to follow when the decision has been made to remove a traffic signal. Those steps are:

1. Determine the appropriate traffic control to be used after removal of the signal.
2. Remove any sight-distance restrictions as necessary.
3. Inform the public of the removal study.
4. Flash or cover the signal heads for a minimum of 90 days, and install the appropriate stop control or other traffic control devices
5. Remove the signal if the engineering data collected during the removal study period confirms that the signal is no longer needed.

This report completed Step 1 by showing the different options for stop-controlled operations. For Step 2, if all-way stop-control is preferred at all intersections no additional sight-distance checks are necessary. If two-way stop-control is desired at any intersection, intersection sight distance needs to be verified before moving forward.

Steps 3 through 5 will be performed in a future project phase. Step 4 alerts drivers that a significant change has occurred with a signal flashing to attract their attention to a changed condition, and allows the City to easily revert to signalized operations if it is shown that the change isn't acceptable. Step 5 exists to confirm the results of this initial study, where conditions may change between the timing of the study and the construction work to remove the signal.

3.5.1 INTERSECTION SIGHT DISTANCE

A key component of a two-way stop-controlled intersection design is assuring that adequate sight distance exists for side street vehicles. Sight distance requirements for TWSC intersections are defined in the Indiana Design Manual (IDM) Chapter 46. Sight distance triangles show all the area that a driver needs to be able to see in order to safely make a left-turn, through, or right-turn from a stopped condition. All the requirements used to build the sight distance triangles at each intersection can be seen in Appendix E. Two-way stop-controlled sight distance triangles were constructed for Jefferson St. at West St., Jefferson St. at Independence St., and Main St. at Madison St.; because of the poor LOS for Jefferson St. at Main shown in Table 4, TWSC isn't a valid option there, therefore sight distance triangles were not constructed.



Figure 5 – Sight Distance Triangles for Northbound West St.



Figure 6 – Sight Distance Triangles for Southbound West St.



Figure 7 – Sight Distance Triangles for Northbound Independence St.

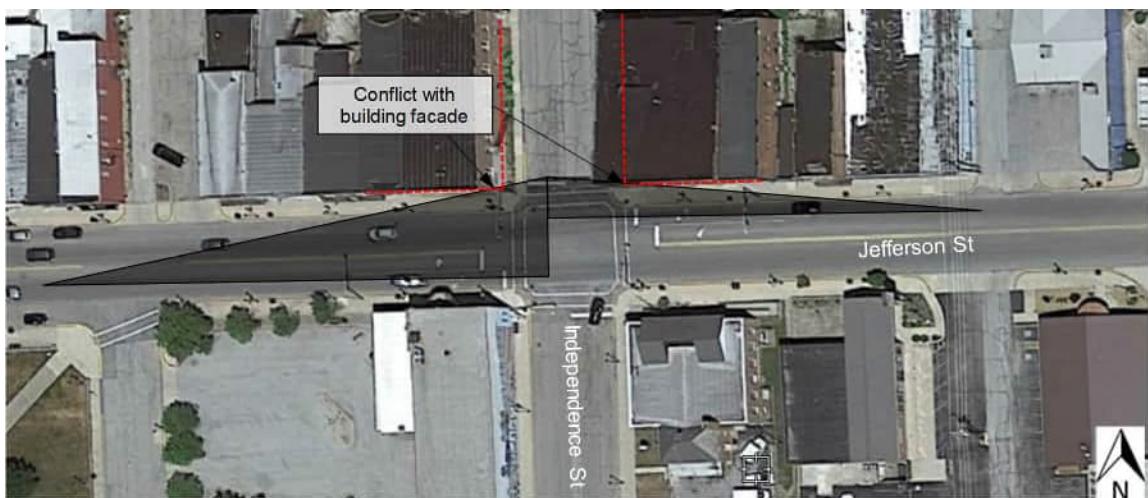


Figure 8 – Sight Distance Triangles for southbound Independence St.



Figure 9 – Sight Distance Triangles for eastbound and westbound Madison St.

Figures 5 & 6 show the sight distance triangles for West St. at Jefferson St. A tree currently obstructs the sight distance for southbound right-turners. Figures 7 & 8 show the sight distance triangles for Independence St. at Jefferson St. Both directions have conflicts where adequate sight distance is unattainable under the current intersection geometry. Similarly Figure 9 shows that westbound right-turners have inadequate sight distance that cannot be fixed with the current geometry.

Figure 6 stated that two-way stop-controlled operations could exist at Jefferson at West if the tree on the northeast corner was either cut down or trimmed, allowing drivers to see past where it currently obstructs the view. TWSC operations could be achieved at Jefferson St. at Independence St. and Main St. at Madison St. if changes to the geometry were constructed. Each intersection has adequate width on the mainline to construct curb bump outs. These bump outs would only require a small length of mainline pavement, meaning they could be built by removing little to no existing parking spaces. Bump outs would allow the side street stop bars to move closer to the center of the mainline, keeping the buildings from obstructing the sight distance triangles. Any future improvements should include a design-level verification of the intersection sight distance necessary at each approach.

4 SUMMARY

INDOT recently relinquished stretches of SR 28 and SR 19 in Tipton, Indiana, which is believed to impact the traffic through the segments now controlled by Tipton. This study was commissioned to examine the existing conditions at the four signalized intersections that are now under the City of Tipton's jurisdiction, determine if the signals are still warranted, and analyze alternate operations if the signals can be removed.

A traffic signal warrant analysis was conducted at each of the four intersections, reviewing the eight-hour signal warrant and the peak hour signal warrant. Conservative traffic volume thresholds were used to ensure the signal warrant analysis considered the worst-case scenario. The results showed that no intersection met either traffic signal warrant.

An analysis was then conducted for changing the intersections to two-way stop controlled (TWSC), all-way stop-controlled (AWSC), and a combination of the two types. To help understand the impact each scenario would have on traffic the anticipated level of service (LOS), delay, and queue length were analyzed at each intersection during the morning (AM) and evening (PM) peak hours of travel for each scenario.

The results showed that TWSC decreased the intersection delay, both compared to existing conditions and to AWSC. However, at the Jefferson St. and Main St. intersection TWSC resulted in unacceptable LOS for Main St. movements. Changing this intersection to AWSC is recommended. At Jefferson at West St., Jefferson St. at Independence St., and Main St. at Madison St. the existing sight distance is inadequate for TWSC. This can be alleviated by trimming a tree at Jefferson St. at West St., and constructing curb bump outs at Jefferson St. at Independence St. and Main St. at Madison St.

The recommended alternative is Scenario 3, combining TWSC and AWSC to achieve the optimal operations through the relinquished corridors. However, if the geometric changes necessary to accommodate TWSC are determined to be infeasible, AWSC is an acceptable alternative at each intersection. Any signal removal should follow the steps from the Indiana Manual on Uniform Traffic Control Devices (IMUTCD) as previously discussed.

APPENDIX A

TRAFFIC DATA

Study Name Jefferson Street at West Street
Project 2023.D056 Tipton, IN
Project Code
Legs and Movements All Processed Legs & Movements
Bin Size 15 minutes
Time Zone America/Indiana/Indianapolis
Start Time 2023-02-22 06:00:00
End Time 2023-02-22 19:00:00
Location Jefferson Street at West Street
Latitude and Longitude 40.282192,-86.042936

AM Peak 7:15 AM - 8:15 AM (0.840)
Midday Peak 11:15 AM - 12:15 PM (0.923)
PM Peak (Overall Peak Hour) 4:30 PM - 5:30 PM (0.941)

Leg Direction	West Southbound							Jefferson Westbound						
				App	Peds CW	Peds CCW				App	Peds CW	Peds CCW		
Start Time	Right	Thru	Left	U-Turn	Total		Right	Thru	Left	U-Turn	Total			
2023-02-22 06:00:00	2	2	1	0	5	0	5	39	0	0	44	0	0	
2023-02-22 06:15:00	1	3	1	0	5	0	0	2	56	2	0	60	0	0
2023-02-22 06:30:00	3	5	2	0	10	0	0	2	78	2	0	82	0	0
2023-02-22 06:45:00	5	0	1	0	6	0	0	1	61	2	0	64	0	0
2023-02-22 07:00:00	0	0	1	0	1	0	0	0	61	3	0	64	0	0
2023-02-22 07:15:00	0	4	1	0	5	0	0	1	73	4	0	78	0	0
2023-02-22 07:30:00	1	7	4	0	12	0	0	2	54	0	0	56	0	0
2023-02-22 07:45:00	1	3	0	0	4	0	0	2	63	3	0	68	0	0
2023-02-22 08:00:00	1	2	0	0	3	0	0	3	54	4	0	61	0	0
2023-02-22 08:15:00	1	2	3	0	6	0	0	3	56	1	0	60	0	0
2023-02-22 08:30:00	2	2	0	0	4	0	0	0	36	2	0	38	0	0
2023-02-22 08:45:00	2	1	1	0	4	0	0	1	51	3	0	55	0	0
2023-02-22 09:00:00	4	4	0	0	8	0	0	1	38	6	0	45	0	0
2023-02-22 09:15:00	0	1	2	0	3	0	0	0	59	8	0	67	0	0
2023-02-22 09:30:00	2	5	1	0	8	0	0	1	51	2	0	54	1	0
2023-02-22 09:45:00	2	3	1	0	6	0	0	2	54	0	0	56	0	0
2023-02-22 10:00:00	2	3	0	0	5	0	0	2	46	3	0	51	0	0
2023-02-22 10:15:00	2	5	1	0	8	0	0	3	39	3	0	45	0	1
2023-02-22 10:30:00	0	1	0	0	1	0	0	2	35	4	0	41	0	0
2023-02-22 10:45:00	2	3	3	0	8	0	0	1	50	9	0	60	0	0
2023-02-22 11:00:00	1	1	2	0	4	0	0	0	47	12	0	59	0	0
2023-02-22 11:15:00	3	4	1	0	8	0	0	3	63	9	0	75	0	0
2023-02-22 11:30:00	2	2	2	0	6	0	0	8	55	7	0	70	1	1
2023-02-22 11:45:00	1	5	4	0	10	0	0	2	54	4	0	60	0	0
2023-02-22 12:00:00	1	2	3	0	6	0	1	0	51	8	0	59	0	0
2023-02-22 12:15:00	0	2	0	0	2	0	1	4	60	2	0	66	0	0
2023-02-22 12:30:00	1	3	1	0	5	0	0	1	52	10	0	63	1	0
2023-02-22 12:45:00	1	2	0	0	3	0	0	2	53	5	0	60	0	2
2023-02-22 13:00:00	1	4	5	0	10	0	0	1	42	6	0	49	0	0
2023-02-22 13:15:00	2	2	4	0	8	0	0	0	38	4	0	42	0	0
2023-02-22 13:30:00	1	1	2	0	4	0	0	1	52	3	0	56	0	0
2023-02-22 13:45:00	0	3	0	0	3	0	0	0	47	2	0	49	0	0
2023-02-22 14:00:00	2	3	1	0	6	0	0	3	43	4	0	50	0	0
2023-02-22 14:15:00	3	1	1	0	5	0	0	0	52	8	0	60	0	0
2023-02-22 14:30:00	1	6	1	0	8	0	1	0	40	7	0	47	1	1
2023-02-22 14:45:00	0	3	0	0	3	0	0	6	49	6	0	61	0	0
2023-02-22 15:00:00	1	2	0	0	3	0	0	6	63	5	0	74	0	0
2023-02-22 15:15:00	1	7	0	0	8	0	2	4	62	3	0	69	1	3
2023-02-22 15:30:00	1	2	1	0	4	0	0	4	60	8	0	72	0	0
2023-02-22 15:45:00	0	4	2	0	6	0	0	3	69	7	0	79	0	0
2023-02-22 16:00:00	3	4	3	0	10	0	0	3	59	9	0	71	0	0
2023-02-22 16:15:00	1	5	1	0	7	0	0	5	60	7	0	72	0	2
2023-02-22 16:30:00	0	5	3	0	8	0	0	4	62	6	0	72	0	0
2023-02-22 16:45:00	0	2	2	0	4	0	0	7	66	12	0	85	0	0
2023-02-22 17:00:00	1	6	2	0	9	0	0	0	78	6	0	84	0	0
2023-02-22 17:15:00	2	1	1	0	4	0	0	9	71	14	0	94	0	0
2023-02-22 17:30:00	1	10	1	0	12	0	0	6	69	5	0	80	0	0
2023-02-22 17:45:00	2	4	1	0	7	0	0	6	44	7	0	57	0	0
2023-02-22 18:00:00	1	5	3	0	9	0	0	3	49	4	0	56	0	0
2023-02-22 18:15:00	0	1	2	0	3	0	1	2	57	5	0	64	0	1
2023-02-22 18:30:00	3	1	1	0	5	1	0	3	34	5	0	42	0	0
2023-02-22 18:45:00	1	2	1	0	4	0	0	1	37	6	0	44	0	0
Grand Total	71	161	74	0	306	1	6	131	2792	267	0	3190	5	11
% Approach	23.2%	52.6%	24.2%	0.0%				4.1%	87.5%	8.4%	0.0%			
% Total	1.0%	2.2%	1.0%	0.0%	4.3%			1.8%	39.0%	3.7%	0.0%	44.5%		
Lights	68	158	74	0	300			128	2519	264	0	2911		
% Lights	95.8%	98.1%	100.0%	0.0%	98.0%			97.7%	90.2%	98.9%	0.0%	91.3%		
Articulated Trucks	1	0	0	0	1			0	211	1	0	212		
% Articulated Trucks	1.4%	0.0%	0.0%	0.0%	0.3%			0.0%	7.6%	0.4%	0.0%	6.6%		
Buses and Single-Unit	2	3	0	0	5			3	62	2	0	67		
% Buses and Single-Unit	2.8%	1.9%	0.0%	0.0%	1.6%			2.3%	2.2%	0.7%	0.0%	2.1%		
Pedestrians						1	6						4	9
% Pedestrians						100.0%	100.0%						80.0%	81.8%
Bicycles on Crosswalk						0	0						1	2
% Bicycles on Crosswalk						0.0%	0.0%						20.0%	18.2%

Start Time	West							Jefferson							
	Northbound			App	Peds	Peds	App	Peds	Peds	Int					
	Right	Thru	Left	U-Turn	Total	CW	CCW	Right	Thru	Left	U-Turn	Total	CW	CCW	Total
2023-02-22 06:00:00	0	2	1	0	3	0	0	0	33	1	0	34	0	0	86
2023-02-22 06:15:00	3	0	1	0	4	0	0	4	22	0	0	26	0	0	95
2023-02-22 06:30:00	0	2	4	0	6	0	0	5	50	0	0	55	0	0	153
2023-02-22 06:45:00	4	1	3	0	8	0	0	2	34	0	0	36	0	0	114
2023-02-22 07:00:00	1	1	2	0	4	0	0	5	32	0	0	37	0	0	106
2023-02-22 07:15:00	2	0	7	0	9	0	0	2	63	1	0	66	0	0	158
2023-02-22 07:30:00	3	1	9	0	13	0	0	6	59	1	0	66	0	0	147
2023-02-22 07:45:00	9	4	4	0	17	0	0	10	80	1	0	91	0	0	180
2023-02-22 08:00:00	2	2	3	0	7	0	0	7	42	0	0	49	0	0	120
2023-02-22 08:15:00	3	3	4	0	10	0	0	1	43	0	0	44	0	0	120
2023-02-22 08:30:00	4	5	2	0	11	0	0	3	48	1	0	52	0	0	105
2023-02-22 08:45:00	2	1	2	0	5	0	0	3	33	0	0	36	0	1	100
2023-02-22 09:00:00	1	0	2	0	3	0	0	3	62	1	0	66	0	0	122
2023-02-22 09:15:00	4	2	6	0	12	0	0	1	43	3	0	47	0	0	129
2023-02-22 09:30:00	6	4	0	0	10	0	0	2	38	0	0	40	0	0	112
2023-02-22 09:45:00	6	2	4	0	12	0	0	2	44	1	0	47	0	0	121
2023-02-22 10:00:00	1	3	4	0	8	0	0	5	47	2	0	54	0	0	118
2023-02-22 10:15:00	4	1	4	0	9	0	0	4	39	1	0	44	0	1	106
2023-02-22 10:30:00	5	2	5	0	12	0	0	3	46	0	0	49	0	0	103
2023-02-22 10:45:00	10	4	2	0	16	0	0	2	27	2	0	31	0	0	115
2023-02-22 11:00:00	5	0	9	0	14	0	0	6	42	0	0	48	0	0	125
2023-02-22 11:15:00	7	2	6	0	15	0	0	6	54	2	0	62	0	0	160
2023-02-22 11:30:00	5	3	6	0	14	0	0	4	59	2	0	65	0	0	155
2023-02-22 11:45:00	5	5	3	0	13	0	0	6	45	1	0	52	0	0	135
2023-02-22 12:00:00	8	1	5	0	14	0	0	5	56	1	0	62	0	1	141
2023-02-22 12:15:00	5	1	8	0	14	0	0	4	43	2	0	49	0	0	131
2023-02-22 12:30:00	9	2	6	0	17	0	0	2	44	2	0	48	0	0	133
2023-02-22 12:45:00	7	4	8	0	19	0	0	2	52	2	0	56	0	0	138
2023-02-22 13:00:00	6	5	5	0	16	0	0	3	43	2	0	48	0	0	123
2023-02-22 13:15:00	4	7	1	0	12	0	0	1	38	1	0	40	0	0	102
2023-02-22 13:30:00	5	1	6	0	12	0	0	4	55	2	0	61	1	0	133
2023-02-22 13:45:00	8	1	3	0	12	0	0	2	44	0	0	46	0	0	110
2023-02-22 14:00:00	8	3	1	0	12	0	0	0	51	1	0	52	0	0	120
2023-02-22 14:15:00	4	3	4	0	11	0	0	3	53	2	0	58	0	0	134
2023-02-22 14:30:00	8	3	3	0	14	0	1	2	59	1	0	62	0	0	131
2023-02-22 14:45:00	5	2	2	0	9	0	0	9	53	0	0	62	0	0	135
2023-02-22 15:00:00	6	2	8	0	16	0	1	2	57	2	0	61	0	0	154
2023-02-22 15:15:00	8	3	10	0	21	0	0	3	84	2	0	89	0	0	187
2023-02-22 15:30:00	5	1	6	0	12	0	0	4	107	3	0	114	0	0	202
2023-02-22 15:45:00	3	1	5	0	9	0	0	6	68	1	0	75	0	0	169
2023-02-22 16:00:00	8	1	5	0	14	0	0	1	74	2	0	77	0	0	172
2023-02-22 16:15:00	3	8	4	0	15	0	2	4	80	1	0	85	3	0	179
2023-02-22 16:30:00	8	3	9	0	20	0	0	5	84	2	0	91	0	0	191
2023-02-22 16:45:00	6	0	3	0	9	0	0	4	85	2	0	91	0	0	189
2023-02-22 17:00:00	11	3	6	0	20	0	0	3	73	3	0	79	0	0	192
2023-02-22 17:15:00	8	3	11	0	22	1	0	5	82	0	0	87	0	0	207
2023-02-22 17:30:00	3	2	8	0	13	0	0	8	64	2	0	74	0	0	179
2023-02-22 17:45:00	8	4	7	0	19	0	0	6	89	3	0	98	0	0	181
2023-02-22 18:00:00	9	4	5	0	18	0	1	5	56	0	0	61	0	0	144
2023-02-22 18:15:00	4	1	2	0	7	0	0	1	36	0	0	37	0	0	111
2023-02-22 18:30:00	6	1	8	0	15	0	0	6	42	0	0	48	0	0	110
2023-02-22 18:45:00	1	1	4	0	6	0	0	3	26	1	0	30	0	0	84
Grand Total	266	121	246	0	633	1	5	195	2783	60	0	3038	4	3	7167
% Approach	42.0%	19.1%	38.9%	0.0%				6.4%	91.6%	2.0%	0.0%				
% Total	3.7%	1.7%	3.4%	0.0%	8.8%			2.7%	38.8%	0.8%	0.0%	42.4%			
Lights	261	119	237	0	617			191	2502	58	0	2751			6579
% Lights	98.1%	98.3%	96.3%	0.0%	97.5%			97.9%	89.9%	96.7%	0.0%	90.6%			91.8%
Articulated Trucks	0	0	2	0	2			1	208	1	0	210			425
% Articulated Trucks	0.0%	0.0%	0.8%	0.0%	0.3%			0.5%	7.5%	1.7%	0.0%	6.9%			5.9%
Buses and Single-Uni	5	2	7	0	14			3	73	1	0	77			163
% Buses and Single-l	1.9%	1.7%	2.8%	0.0%	2.2%			1.5%	2.6%	1.7%	0.0%	2.5%			2.3%
Pedestrians						1	5						1	2	
% Pedestrians						#####	#####						25.0%	66.7%	
Bicycles on Crosswalk						0	0						3	1	
% Bicycles on Crosswalk						0.0%	0.0%						75.0%	33.3%	

Study Name Jefferson Street at Main Street
Project 2023.D056 Tipton, IN
Project Code
Legs and Movements All Processed Legs & Movements
Bin Size 15 minutes
Time Zone America/Indiana/Indianapolis
Start Time 2023-02-22 06:00:00
End Time 2023-02-22 19:00:00
Location Jefferson Street at Main Street
Latitude and Longitude 40.282213,-86.041182

AM Peak 7:15 AM - 8:15 AM (0.828)
Midday Peak 11:15 AM - 12:15 PM (0.966)
PM Peak (Overall Peak Hour) 4:30 PM - 5:30 PM (0.975)

Leg	Main							Jefferson						
Direction	Southbound							Westbound						
Start Time	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW
2023-02-22 06:00:00	6	1	4	0	11	0	0	4	38	7	0	49	0	0
2023-02-22 06:15:00	1	3	3	0	7	0	0	3	54	11	0	68	0	0
2023-02-22 06:30:00	4	7	1	0	12	0	0	3	74	17	0	94	0	0
2023-02-22 06:45:00	5	6	2	0	13	0	0	8	54	10	0	72	0	0
2023-02-22 07:00:00	5	13	6	0	24	0	0	1	58	18	0	77	0	0
2023-02-22 07:15:00	5	19	3	0	27	0	0	6	67	23	0	96	0	0
2023-02-22 07:30:00	5	46	4	0	55	0	0	2	49	34	0	85	0	0
2023-02-22 07:45:00	4	33	5	0	42	0	0	6	53	20	0	79	0	0
2023-02-22 08:00:00	7	19	4	0	30	0	0	4	45	20	0	69	0	0
2023-02-22 08:15:00	7	15	6	0	28	0	0	4	51	12	0	67	0	0
2023-02-22 08:30:00	3	16	4	0	23	0	0	4	25	15	0	44	0	0
2023-02-22 08:45:00	3	9	10	0	22	0	0	7	51	25	0	83	1	1
2023-02-22 09:00:00	3	10	7	0	20	0	0	7	44	13	0	64	0	0
2023-02-22 09:15:00	2	13	10	0	25	0	0	7	57	17	0	81	0	1
2023-02-22 09:30:00	2	11	7	0	20	0	0	4	53	9	0	66	0	0
2023-02-22 09:45:00	2	10	5	0	17	0	0	10	46	24	0	80	0	0
2023-02-22 10:00:00	4	12	10	0	26	0	0	6	40	18	0	64	0	0
2023-02-22 10:15:00	3	13	4	0	20	0	0	8	38	17	0	63	0	0
2023-02-22 10:30:00	3	10	5	0	18	0	0	8	36	14	0	58	0	1
2023-02-22 10:45:00	5	13	4	0	22	0	1	7	53	13	0	73	0	0
2023-02-22 11:00:00	5	12	6	0	23	0	1	9	45	10	0	64	0	0
2023-02-22 11:15:00	5	12	9	0	26	0	0	12	61	12	0	85	0	0
2023-02-22 11:30:00	9	12	4	0	25	0	2	4	52	21	0	77	0	0
2023-02-22 11:45:00	7	13	2	0	22	0	0	5	47	19	0	71	0	0
2023-02-22 12:00:00	5	12	8	0	25	2	0	3	50	16	0	69	0	0
2023-02-22 12:15:00	3	14	9	0	26	0	0	0	53	20	0	73	0	0
2023-02-22 12:30:00	6	18	9	0	33	0	0	5	57	18	0	80	0	0
2023-02-22 12:45:00	7	14	4	0	25	0	0	11	48	21	0	80	0	0
2023-02-22 13:00:00	4	13	5	0	22	1	0	8	35	15	0	58	0	0
2023-02-22 13:15:00	4	15	12	0	31	0	1	2	40	12	0	54	0	0
2023-02-22 13:30:00	7	12	3	0	22	0	0	5	47	19	0	71	0	0
2023-02-22 13:45:00	2	10	7	0	19	0	0	2	46	13	0	61	0	0
2023-02-22 14:00:00	3	5	6	0	14	0	1	11	37	13	0	61	0	0
2023-02-22 14:15:00	6	13	7	0	26	2	2	3	50	20	0	73	0	0
2023-02-22 14:30:00	6	17	5	0	28	1	0	10	44	20	0	74	0	2
2023-02-22 14:45:00	6	15	7	0	28	1	1	5	50	19	0	74	0	0
2023-02-22 15:00:00	2	5	6	0	13	0	0	10	57	10	0	77	0	0
2023-02-22 15:15:00	6	15	10	0	31	0	1	5	53	18	0	76	0	2
2023-02-22 15:30:00	7	10	13	0	30	2	0	8	47	19	0	74	1	1
2023-02-22 15:45:00	5	10	5	0	20	0	0	11	57	13	0	81	1	0
2023-02-22 16:00:00	4	12	3	0	19	0	1	4	54	16	0	74	0	0
2023-02-22 16:15:00	3	14	8	0	25	1	0	3	59	17	0	79	0	0
2023-02-22 16:30:00	4	16	9	0	29	1	0	13	58	18	0	89	0	0
2023-02-22 16:45:00	6	16	6	0	28	1	0	10	67	16	0	93	0	0
2023-02-22 17:00:00	3	25	4	0	32	0	1	3	64	12	0	79	0	0
2023-02-22 17:15:00	6	14	3	0	23	0	1	6	70	20	0	96	0	0
2023-02-22 17:30:00	2	16	5	0	23	0	1	12	57	23	0	92	1	1
2023-02-22 17:45:00	3	24	3	0	30	0	0	10	44	21	0	75	0	0
2023-02-22 18:00:00	0	9	4	0	13	1	0	0	44	9	0	53	0	0
2023-02-22 18:15:00	5	5	7	0	17	0	0	8	53	16	0	77	0	0
2023-02-22 18:30:00	3	3	6	0	12	1	1	5	32	4	0	41	0	0
2023-02-22 18:45:00	4	4	5	0	13	2	0	1	32	15	0	48	0	0
Grand Total	227	684	304	0	1215	16	15	313	2596	852	0	3761	4	9
% Approach	18.7%	56.3%	25.0%	0.0%				8.3%	69.0%	22.7%	0.0%			
% Total	2.2%	6.8%	3.0%	0.0%	12.0%			3.1%	25.7%	8.4%	0.0%	37.2%		
Lights	220	669	298	0	1187			307	2337	832	0	3476		
% Lights	96.9%	97.8%	98.0%	0.0%	97.7%			98.1%	90.0%	97.7%	0.0%	92.4%		
Articulated Trucks	2	1	1	0	4			0	209	10	0	219		
% Articulated Trucks	0.9%	0.1%	0.3%	0.0%	0.3%			0.0%	8.1%	1.2%	0.0%	5.8%		
Buses and Single-Un	5	14	5	0	24			6	50	10	0	66		
% Buses and Single-U	2.2%	2.0%	1.6%	0.0%	2.0%			1.9%	1.9%	1.2%	0.0%	1.8%		
Pedestrians						16	15					4	9	
% Pedestrians						100.0%	100.0%					100.0%	100.0%	
Bicycles on Crosswalk						0	0					0	0	
% Bicycles on Crosswalk						0.0%	0.0%					0.0%	0.0%	

Start Time	Main								Jefferson							
	Northbound								Eastbound							
	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Int Total	
2023-02-22 06:00:00	5	1	2	0	8	0	0	8	25	1	0	34	0	0	102	
2023-02-22 06:15:00	2	5	3	0	10	0	0	8	17	1	0	26	0	0	111	
2023-02-22 06:30:00	1	7	3	0	11	0	0	4	46	3	0	53	0	0	170	
2023-02-22 06:45:00	10	10	6	0	26	0	0	8	31	2	0	41	0	0	152	
2023-02-22 07:00:00	6	6	4	0	16	0	0	12	25	1	0	38	0	0	155	
2023-02-22 07:15:00	10	8	5	0	23	0	0	9	54	2	0	65	0	0	211	
2023-02-22 07:30:00	11	10	5	0	26	0	0	24	38	1	0	63	0	1	229	
2023-02-22 07:45:00	27	24	15	0	66	0	0	27	51	8	0	86	0	0	273	
2023-02-22 08:00:00	18	16	7	0	41	0	0	6	42	3	0	51	0	0	191	
2023-02-22 08:15:00	13	11	4	0	28	0	0	5	38	2	0	45	0	0	168	
2023-02-22 08:30:00	8	13	5	0	26	0	0	8	43	1	0	52	0	0	145	
2023-02-22 08:45:00	17	8	2	0	27	0	0	4	29	5	0	38	0	0	170	
2023-02-22 09:00:00	15	3	6	0	24	0	0	11	49	4	0	64	0	0	172	
2023-02-22 09:15:00	17	11	8	0	36	0	0	5	42	3	0	50	0	0	192	
2023-02-22 09:30:00	11	11	5	0	27	0	0	8	34	5	0	47	0	0	160	
2023-02-22 09:45:00	20	10	8	0	38	0	0	2	44	4	0	50	0	0	185	
2023-02-22 10:00:00	7	17	6	0	30	0	0	2	44	1	0	47	0	0	167	
2023-02-22 10:15:00	7	7	8	0	22	0	0	10	28	1	0	39	0	0	144	
2023-02-22 10:30:00	15	9	5	0	29	0	1	7	42	3	0	52	0	0	157	
2023-02-22 10:45:00	15	9	5	0	29	0	0	6	32	1	0	39	0	2	163	
2023-02-22 11:00:00	14	12	13	0	39	0	0	6	38	5	0	49	1	1	175	
2023-02-22 11:15:00	19	5	6	0	30	1	0	5	52	6	0	63	1	0	204	
2023-02-22 11:30:00	15	16	6	0	37	0	0	7	57	8	0	72	1	0	211	
2023-02-22 11:45:00	15	16	6	0	37	0	0	8	51	3	0	62	0	0	192	
2023-02-22 12:00:00	19	12	5	0	36	0	0	8	66	4	0	78	0	0	208	
2023-02-22 12:15:00	16	10	7	0	33	0	0	3	39	8	0	50	0	0	182	
2023-02-22 12:30:00	20	14	6	0	40	0	0	6	43	5	0	54	0	0	207	
2023-02-22 12:45:00	7	8	11	0	26	0	0	10	49	9	0	68	0	0	199	
2023-02-22 13:00:00	19	9	8	0	36	0	0	12	42	4	0	58	0	0	174	
2023-02-22 13:15:00	12	10	3	0	25	0	0	7	46	0	0	53	0	0	163	
2023-02-22 13:30:00	8	7	4	0	19	0	0	9	49	2	0	60	0	0	172	
2023-02-22 13:45:00	13	5	4	0	22	0	0	15	39	6	0	60	0	0	162	
2023-02-22 14:00:00	19	12	9	0	40	0	0	5	59	4	0	68	0	0	183	
2023-02-22 14:15:00	14	9	5	0	28	0	0	5	62	4	0	71	0	0	198	
2023-02-22 14:30:00	8	10	6	0	24	0	0	8	61	2	0	71	0	0	197	
2023-02-22 14:45:00	12	15	4	0	31	0	0	13	45	8	0	66	0	0	199	
2023-02-22 15:00:00	23	34	21	0	78	0	0	9	58	8	0	75	0	0	243	
2023-02-22 15:15:00	25	18	13	0	56	1	0	13	75	5	0	93	0	0	256	
2023-02-22 15:30:00	15	23	11	0	49	0	0	14	105	6	0	125	0	0	278	
2023-02-22 15:45:00	23	12	16	0	51	1	0	9	64	7	0	80	0	0	232	
2023-02-22 16:00:00	21	19	17	0	57	0	0	11	74	6	0	91	0	0	241	
2023-02-22 16:15:00	20	18	13	0	51	0	0	7	81	4	0	92	1	0	247	
2023-02-22 16:30:00	25	24	10	0	59	0	0	13	77	4	0	94	1	0	271	
2023-02-22 16:45:00	20	8	11	0	39	0	0	9	80	11	0	100	0	0	260	
2023-02-22 17:00:00	19	25	18	0	62	0	0	11	80	8	0	99	0	0	272	
2023-02-22 17:15:00	32	13	19	0	64	0	0	5	86	3	0	94	0	0	277	
2023-02-22 17:30:00	21	7	13	0	41	0	0	12	60	3	0	75	0	0	231	
2023-02-22 17:45:00	19	23	7	0	49	0	0	17	83	3	0	103	2	0	257	
2023-02-22 18:00:00	17	14	12	0	43	0	0	9	58	6	0	73	0	0	182	
2023-02-22 18:15:00	12	9	7	0	28	0	0	6	37	4	0	47	0	0	169	
2023-02-22 18:30:00	11	8	7	0	26	0	0	11	40	4	0	55	0	0	134	
2023-02-22 18:45:00	15	10	7	0	32	0	0	4	26	1	0	31	0	0	124	
Grand Total	783	631	417	0	1831	3	1	461	2636	213	0	3310	7	4	10117	
% Approach	42.8%	34.5%	22.8%	0.0%				13.9%	79.6%	6.4%	0.0%					
% Total	7.7%	6.2%	4.1%	0.0%	18.1%			4.6%	26.1%	2.1%	0.0%	32.7%				
Lights	752	613	396	0	1761			438	2381	206	0	3025			9449	
% Lights	96.0%	97.1%	95.0%	0.0%	96.2%			95.0%	90.3%	96.7%	0.0%	91.4%			93.4%	
Articulated Trucks	11	2	7	0	20			6	200	3	0	209			452	
% Articulated Trucks	1.4%	0.3%	1.7%	0.0%	1.1%			1.3%	7.6%	1.4%	0.0%	6.3%			4.5%	
Buses and Single-Unit	20	16	14	0	50			17	55	4	0	76			216	
% Buses and Single-U	2.6%	2.5%	3.4%	0.0%	2.7%			3.7%	2.1%	1.9%	0.0%	2.3%			2.1%	
Pedestrians						3	1								6	4
% Pedestrians						#####	#####								85.7%	#####
Bicycles on Crosswalk						0	0								1	0
% Bicycles on Crosswalk						0.0%	0.0%								14.3%	0.0%

Study Name Jefferson Street at Independence Street
Project 2023.D056 Tipton, IN
Project Code
Legs and Movements All Processed Legs & Movements
Bin Size 15 minutes
Time Zone America/Indiana/Indianapolis
Start Time 2023-02-22 06:00:00
End Time 2023-02-22 19:00:00
Location Jefferson Street at Independence Street
Latitude and Longitude 40.282287,-86.039497

AM Peak 7:15 AM - 8:15 AM (0.857)
Midday Peak 11:30 AM - 12:30 PM (0.942)
PM Peak (Overall Peak Hour) 4:30 PM - 5:30 PM (0.903)

Leg Direction	Independence Southbound							Jefferson Westbound						
	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW
Start Time	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW
2023-02-22 06:00:00	2	0	1	0	3	0	0	1	45	1	0	47	0	0
2023-02-22 06:15:00	5	1	2	0	8	0	0	0	66	0	0	66	0	0
2023-02-22 06:30:00	4	1	0	0	5	0	0	2	86	3	0	91	0	0
2023-02-22 06:45:00	3	1	2	0	6	0	0	4	69	2	0	75	0	0
2023-02-22 07:00:00	3	3	4	0	10	0	0	3	75	5	0	83	0	0
2023-02-22 07:15:00	9	0	0	0	9	0	0	0	88	6	0	94	0	0
2023-02-22 07:30:00	1	5	3	0	9	0	0	2	88	2	0	92	0	0
2023-02-22 07:45:00	3	5	5	0	13	0	0	8	83	4	0	95	0	0
2023-02-22 08:00:00	3	4	1	0	8	0	0	4	66	5	0	75	0	0
2023-02-22 08:15:00	4	2	1	0	7	0	0	2	66	6	0	74	0	0
2023-02-22 08:30:00	1	2	1	0	4	0	0	6	38	5	0	49	0	0
2023-02-22 08:45:00	5	4	3	0	12	0	0	5	80	3	0	88	1	0
2023-02-22 09:00:00	0	1	5	0	6	0	1	4	64	1	0	69	0	1
2023-02-22 09:15:00	2	1	2	0	5	0	0	2	80	6	0	88	0	0
2023-02-22 09:30:00	3	3	3	0	9	0	0	3	63	4	0	70	0	0
2023-02-22 09:45:00	2	4	3	0	9	0	0	5	88	4	0	97	0	0
2023-02-22 10:00:00	2	3	5	0	10	0	0	7	60	3	0	70	0	0
2023-02-22 10:15:00	1	4	3	0	8	0	0	1	60	5	0	66	0	0
2023-02-22 10:30:00	3	3	1	0	7	0	0	4	60	2	0	66	0	0
2023-02-22 10:45:00	5	3	3	0	11	0	0	6	66	5	0	77	1	0
2023-02-22 11:00:00	2	3	5	0	10	0	0	5	61	2	0	68	0	0
2023-02-22 11:15:00	1	2	3	0	6	0	0	0	79	1	0	80	0	0
2023-02-22 11:30:00	2	3	3	0	8	0	0	3	78	3	0	84	0	0
2023-02-22 11:45:00	4	3	7	0	14	0	0	5	70	5	0	80	0	0
2023-02-22 12:00:00	4	4	3	0	11	0	0	6	64	7	0	77	1	0
2023-02-22 12:15:00	7	4	5	0	16	0	1	7	72	3	0	82	0	1
2023-02-22 12:30:00	7	1	4	0	12	0	0	8	75	10	0	93	0	0
2023-02-22 12:45:00	3	4	2	0	9	1	0	5	81	8	0	94	0	0
2023-02-22 13:00:00	3	7	7	0	17	0	1	4	57	3	0	64	0	0
2023-02-22 13:15:00	3	3	7	0	13	0	0	4	51	4	0	59	0	0
2023-02-22 13:30:00	1	5	5	0	11	0	0	7	67	6	0	80	0	0
2023-02-22 13:45:00	3	2	5	0	10	0	0	3	58	6	0	67	1	0
2023-02-22 14:00:00	4	3	6	0	13	0	0	3	58	4	0	65	0	0
2023-02-22 14:15:00	2	1	4	0	7	0	0	2	70	2	0	74	0	0
2023-02-22 14:30:00	4	4	3	0	11	0	1	5	71	5	0	81	0	0
2023-02-22 14:45:00	4	0	2	0	6	1	0	3	65	4	0	72	0	0
2023-02-22 15:00:00	3	4	4	0	11	0	0	3	77	7	0	87	0	0
2023-02-22 15:15:00	0	7	4	0	11	0	1	5	75	3	0	83	0	0
2023-02-22 15:30:00	3	3	2	0	8	0	0	4	73	7	0	84	0	0
2023-02-22 15:45:00	3	11	6	0	20	1	1	6	73	2	0	81	0	0
2023-02-22 16:00:00	7	4	4	0	15	0	0	5	72	4	0	81	0	0
2023-02-22 16:15:00	4	4	2	0	10	0	0	3	78	2	0	83	0	0
2023-02-22 16:30:00	7	4	2	0	13	2	0	6	80	5	0	91	1	0
2023-02-22 16:45:00	6	2	0	0	8	0	0	5	94	2	0	101	0	0
2023-02-22 17:00:00	4	4	3	0	11	1	0	6	76	3	0	85	0	0
2023-02-22 17:15:00	7	4	5	0	16	0	0	7	86	5	0	98	0	0
2023-02-22 17:30:00	3	4	5	0	12	0	0	4	89	7	0	100	0	2
2023-02-22 17:45:00	4	1	3	0	8	0	0	3	70	4	0	77	0	0
2023-02-22 18:00:00	8	2	2	0	12	0	0	3	51	4	0	58	0	0
2023-02-22 18:15:00	1	1	5	0	7	0	0	1	78	6	0	85	0	0
2023-02-22 18:30:00	1	3	1	0	5	1	0	5	36	2	0	43	0	0
2023-02-22 18:45:00	0	3	2	0	5	1	0	1	48	4	0	53	0	0
Grand Total	176	160	169	0	505	8	6	206	3624	212	0	4042	5	4
% Approach	34.9%	31.7%	33.5%	0.0%				5.1%	89.7%	5.2%	0.0%			
% Total	2.0%	1.8%	1.9%	0.0%	5.8%			2.4%	41.7%	2.4%	0.0%	46.5%		
Lights	174	158	164	0	496			203	3350	208	0	3761		
% Lights	98.9%	98.8%	97.0%	0.0%	98.2%			98.5%	92.4%	98.1%	0.0%	93.0%		
Articulated Trucks	0	1	2	0	3			0	219	0	0	219		
% Articulated Trucks	0.0%	0.6%	1.2%	0.0%	0.6%			0.0%	6.0%	0.0%	0.0%	5.4%		
Buses and Single-Unit	2	1	3	0	6			3	55	4	0	62		
% Buses and Single-Unit	1.1%	0.6%	1.8%	0.0%	1.2%			1.5%	1.5%	1.9%	0.0%	1.5%		
Pedestrians						8	6						4	4
% Pedestrians						100.0%	100.0%						80.0%	100.0%
Bicycles on Crosswalk						0	0						1	0
% Bicycles on Crosswalk						0.0%	0.0%						20.0%	0.0%

Leg Direction	Independence Northbound							Jefferson Eastbound							
	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Int Total
Start Time															
2023-02-22 06:00:00	1	0	1	0	2	0	0	2	31	1	0	34	0	0	86
2023-02-22 06:15:00	0	1	0	0	1	0	0	3	18	1	0	22	0	0	97
2023-02-22 06:30:00	1	0	0	0	1	0	1	0	47	1	0	48	0	0	145
2023-02-22 06:45:00	0	0	1	0	1	0	0	0	40	1	0	41	0	0	123
2023-02-22 07:00:00	11	2	1	0	14	0	0	2	34	0	0	36	0	1	143
2023-02-22 07:15:00	1	2	2	0	5	0	0	1	63	0	0	64	0	0	172
2023-02-22 07:30:00	3	1	0	0	4	1	0	2	46	0	0	48	0	0	153
2023-02-22 07:45:00	4	3	4	0	11	0	0	2	72	3	0	77	0	0	196
2023-02-22 08:00:00	5	2	0	0	7	0	0	1	58	2	0	61	0	0	151
2023-02-22 08:15:00	3	1	2	0	6	0	0	2	48	4	0	54	0	0	141
2023-02-22 08:30:00	2	2	3	0	7	1	0	1	51	1	0	53	0	0	113
2023-02-22 08:45:00	5	4	4	0	13	1	0	4	38	2	0	44	0	0	157
2023-02-22 09:00:00	6	3	4	0	13	0	0	6	58	4	0	68	0	0	156
2023-02-22 09:15:00	8	1	0	0	9	1	3	3	58	0	0	61	0	0	163
2023-02-22 09:30:00	9	0	4	0	13	1	0	2	49	0	0	51	0	0	143
2023-02-22 09:45:00	11	1	1	0	13	0	0	5	57	0	0	62	0	1	181
2023-02-22 10:00:00	6	0	2	0	8	0	0	0	55	3	0	58	0	0	146
2023-02-22 10:15:00	7	2	3	0	12	1	0	2	39	1	0	42	0	0	128
2023-02-22 10:30:00	10	3	2	0	15	0	0	3	58	1	0	62	0	0	150
2023-02-22 10:45:00	4	2	0	0	6	0	0	2	47	2	0	51	1	0	145
2023-02-22 11:00:00	9	0	3	0	12	0	0	2	50	0	0	52	0	0	142
2023-02-22 11:15:00	3	2	5	0	10	1	0	7	64	2	0	73	0	0	169
2023-02-22 11:30:00	14	3	5	0	22	0	0	1	71	2	0	74	0	0	188
2023-02-22 11:45:00	14	1	5	0	20	0	0	3	58	2	0	63	0	0	177
2023-02-22 12:00:00	11	4	3	0	18	0	0	4	83	2	0	89	0	0	195
2023-02-22 12:15:00	6	4	3	0	13	0	0	3	61	0	0	64	0	0	175
2023-02-22 12:30:00	11	2	3	0	16	0	0	1	66	0	0	67	0	0	188
2023-02-22 12:45:00	11	3	4	0	18	0	0	3	52	1	0	56	0	0	177
2023-02-22 13:00:00	8	5	2	0	15	0	0	2	52	7	0	61	0	0	157
2023-02-22 13:15:00	9	4	3	0	16	0	0	1	64	2	0	67	0	0	155
2023-02-22 13:30:00	5	5	3	0	13	0	0	3	55	0	0	58	0	0	162
2023-02-22 13:45:00	10	0	1	0	11	0	0	3	43	6	0	52	0	0	140
2023-02-22 14:00:00	2	4	1	0	7	0	0	4	70	2	0	76	0	0	161
2023-02-22 14:15:00	4	3	2	0	9	0	0	3	73	3	0	79	1	0	169
2023-02-22 14:30:00	5	1	5	0	11	0	0	2	69	1	0	72	0	1	175
2023-02-22 14:45:00	7	2	4	0	13	0	0	3	56	2	0	61	0	0	152
2023-02-22 15:00:00	9	3	3	0	15	1	0	5	80	2	0	87	0	0	200
2023-02-22 15:15:00	11	10	1	0	22	0	0	1	103	1	0	105	0	0	221
2023-02-22 15:30:00	9	3	1	0	13	0	0	1	118	3	0	122	0	0	227
2023-02-22 15:45:00	10	7	3	0	20	1	0	4	90	2	0	96	0	0	217
2023-02-22 16:00:00	17	5	1	0	23	1	0	4	89	5	0	98	1	0	217
2023-02-22 16:15:00	6	1	2	0	9	0	0	0	98	4	0	102	0	0	204
2023-02-22 16:30:00	5	3	4	0	12	0	0	5	98	2	0	105	0	0	221
2023-02-22 16:45:00	10	3	2	0	15	1	0	4	92	8	0	104	0	0	228
2023-02-22 17:00:00	8	3	3	0	14	0	0	3	95	1	0	99	0	0	209
2023-02-22 17:15:00	9	1	4	0	14	0	0	2	116	6	0	124	0	0	252
2023-02-22 17:30:00	10	4	3	0	17	0	0	3	75	3	0	81	0	0	210
2023-02-22 17:45:00	8	3	3	0	14	0	0	1	89	6	0	96	0	0	195
2023-02-22 18:00:00	7	1	1	0	9	0	0	0	74	2	0	76	0	0	155
2023-02-22 18:15:00	4	0	4	0	8	0	0	1	52	4	0	57	0	0	157
2023-02-22 18:30:00	1	1	1	0	3	0	0	0	55	0	0	55	0	0	106
2023-02-22 18:45:00	6	1	3	0	10	0	0	1	39	3	0	43	0	0	111
Grand Total	356	122	125	0	603	11	4	123	3317	111	0	3551	3	3	8701
% Approach	59.0%	20.2%	20.7%	0.0%				3.5%	93.4%	3.1%	0.0%				
% Total	4.1%	1.4%	1.4%	0.0%	6.9%			1.4%	38.1%	1.3%	0.0%	40.8%			
Lights	352	115	121	0	588			119	3036	110	0	3265			8110
% Lights	98.9%	94.3%	96.8%	0.0%	97.5%			96.7%	91.5%	99.1%	0.0%	91.9%			93.2%
Articulated Trucks	1	5	0	0	6			0	210	0	0	210			438
% Articulated Trucks	0.3%	4.1%	0.0%	0.0%	1.0%			0.0%	6.3%	0.0%	0.0%	5.9%			5.0%
Buses and Single-Unit	3	2	4	0	9			4	71	1	0	76			153
% Buses and Single-Unit	0.8%	1.6%	3.2%	0.0%	1.5%			3.3%	2.1%	0.9%	0.0%	2.1%			1.8%
Pedestrians						11	4						3	2	
% Pedestrians								100.0%	100.0%				100.0%	66.7%	
Bicycles on Crosswalk								0	0				0	1	
% Bicycles on Crosswalk								0.0%	0.0%				0.0%	33.3%	

Study Name Main Street at Madison Street
Project 2023.D056 Tipton, IN
Project Code
Legs and Movements All Processed Legs & Movements
Bin Size 15 minutes
Time Zone America/Indiana/Indianapolis
Start Time 2023-02-22 06:00:00
End Time 2023-02-22 19:00:00
Location Main Street at Madison Street
Latitude and Longitude 40.280944,-86.041166

AM Peak 7:15 AM - 8:15 AM (0.735)
Midday Peak 11:15 AM - 12:15 PM (0.924)
PM Peak (Overall Peak Hour) 5 PM - 6 PM (0.943)

Leg Direction	Main Southbound								Madison Westbound							
	App			Peds CW	Peds CCW	App			Peds CW	Peds CCW	App					
Start Time	Right	Thru	Left	U-Turn	Total				Right	Thru	Left	U-Turn	Total			
2023-02-22 06:00:00	0	16	0	0	16	0	0	0	1	1	0	2	0	0	0	0
2023-02-22 06:15:00	1	20	0	0	21	0	0	0	0	1	0	1	0	0	1	0
2023-02-22 06:30:00	0	28	0	0	28	0	0	0	0	0	0	0	0	0	0	0
2023-02-22 06:45:00	0	23	1	0	24	0	0	1	0	0	0	1	0	0	0	0
2023-02-22 07:00:00	0	42	1	0	43	0	0	1	1	1	0	3	0	0	0	0
2023-02-22 07:15:00	0	48	1	0	49	0	0	0	3	0	3	0	6	0	0	0
2023-02-22 07:30:00	4	102	1	0	107	0	0	2	2	4	0	8	0	0	0	0
2023-02-22 07:45:00	6	73	1	0	80	0	0	0	1	6	0	7	0	0	0	0
2023-02-22 08:00:00	3	41	2	0	46	0	0	2	1	0	0	3	0	0	0	0
2023-02-22 08:15:00	1	30	2	0	33	0	0	2	1	2	0	5	0	0	0	0
2023-02-22 08:30:00	1	34	3	0	38	0	0	1	1	1	0	3	0	0	0	0
2023-02-22 08:45:00	1	27	2	0	30	0	0	4	2	0	0	6	0	0	0	0
2023-02-22 09:00:00	2	27	3	0	32	0	0	1	0	1	0	2	0	0	0	0
2023-02-22 09:15:00	2	33	2	0	37	0	0	4	0	4	0	8	0	0	0	0
2023-02-22 09:30:00	3	20	2	0	25	0	0	2	5	2	0	9	0	0	0	0
2023-02-22 09:45:00	5	26	4	0	35	0	2	4	5	2	0	11	0	1	0	0
2023-02-22 10:00:00	1	25	2	0	28	0	0	3	0	1	0	4	0	0	0	0
2023-02-22 10:15:00	2	30	3	0	35	0	0	5	2	5	0	12	0	0	0	0
2023-02-22 10:30:00	2	27	1	0	30	0	0	4	3	1	0	8	0	0	0	0
2023-02-22 10:45:00	2	23	2	0	27	0	0	0	2	0	0	2	0	0	0	0
2023-02-22 11:00:00	2	19	2	0	23	0	0	2	3	0	0	5	0	0	0	0
2023-02-22 11:15:00	2	24	5	0	31	0	2	4	3	4	0	11	0	0	0	0
2023-02-22 11:30:00	3	30	3	0	36	1	1	2	0	3	0	5	0	0	0	0
2023-02-22 11:45:00	2	28	4	0	34	0	0	3	3	3	0	9	0	0	0	0
2023-02-22 12:00:00	1	31	1	0	33	0	0	3	2	4	0	9	0	0	0	0
2023-02-22 12:15:00	3	32	0	0	35	0	0	3	1	1	0	5	0	0	0	0
2023-02-22 12:30:00	2	36	4	0	42	0	0	1	0	4	0	5	0	0	0	0
2023-02-22 12:45:00	8	37	1	0	46	1	0	1	2	2	0	5	0	0	0	0
2023-02-22 13:00:00	3	24	4	0	31	0	0	4	1	0	0	5	0	0	0	0
2023-02-22 13:15:00	0	30	2	0	32	0	0	2	1	1	0	4	0	0	0	0
2023-02-22 13:30:00	1	32	2	0	35	0	0	2	2	1	0	5	0	0	0	0
2023-02-22 13:45:00	2	34	2	0	38	0	0	2	1	0	0	3	0	0	0	0
2023-02-22 14:00:00	2	21	1	0	24	0	0	0	1	3	0	4	0	0	0	0
2023-02-22 14:15:00	2	33	1	0	36	1	0	1	0	3	0	4	0	0	0	0
2023-02-22 14:30:00	7	37	1	0	45	0	0	3	3	2	0	8	0	1	0	0
2023-02-22 14:45:00	0	46	2	0	48	0	0	3	2	1	0	6	0	0	0	0
2023-02-22 15:00:00	1	23	0	0	24	0	1	2	2	1	0	5	0	0	0	0
2023-02-22 15:15:00	6	42	0	0	48	0	0	7	5	2	0	14	0	3	0	0
2023-02-22 15:30:00	3	33	4	0	40	0	0	4	2	3	0	9	0	0	0	0
2023-02-22 15:45:00	0	29	3	0	32	0	0	4	2	2	0	8	1	0	0	0
2023-02-22 16:00:00	5	34	1	0	40	0	0	8	3	4	0	15	0	0	0	0
2023-02-22 16:15:00	4	28	3	1	36	0	0	1	2	1	0	4	0	0	0	0
2023-02-22 16:30:00	9	34	3	0	46	0	0	5	1	4	0	10	0	0	0	0
2023-02-22 16:45:00	2	34	3	0	39	0	0	0	1	3	0	4	0	0	0	0
2023-02-22 17:00:00	2	35	5	0	42	0	0	3	1	2	0	6	0	0	0	0
2023-02-22 17:15:00	6	34	3	0	43	0	0	1	1	3	0	5	0	0	0	0
2023-02-22 17:30:00	4	41	4	0	49	0	0	2	0	4	0	6	1	1	0	0
2023-02-22 17:45:00	2	53	6	0	61	0	0	1	1	2	0	4	0	0	0	0
2023-02-22 18:00:00	2	23	3	0	28	1	0	1	3	5	0	9	0	0	0	0
2023-02-22 18:15:00	3	16	5	0	24	0	0	0	0	1	0	1	0	0	0	0
2023-02-22 18:30:00	1	15	0	0	16	0	0	2	2	1	0	5	0	0	0	0
2023-02-22 18:45:00	1	21	2	0	24	0	0	2	0	3	0	5	0	0	0	0
Grand Total	127	1684	113	1	1925	4	6	118	78	108	0	304	2	7		
% Approach	6.6%	87.5%	5.9%	0.1%				38.8%	25.7%	35.5%	0.0%					
% Total	2.7%	35.2%	2.4%	0.0%	40.3%			2.5%	1.6%	2.3%	0.0%	6.4%				
Lights	123	1626	112	1	1862			113	77	104	0	294				
% Lights	96.9%	96.6%	99.1%	100.0%	96.7%			95.8%	98.7%	96.3%	0.0%	96.7%				
Articulated Trucks	1	19	1	0	21			0	0	0	0	0	0	0	0	0
% Articulated Trucks	0.8%	1.1%	0.9%	0.0%	1.1%			0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%
Buses and Single-Unit	3	39	0	0	42			5	1	4	0	10				
% Buses and Single-Unit	2.4%	2.3%	0.0%	0.0%	2.2%			4.2%	1.3%	3.7%	0.0%	3.3%				
Pedestrians						4	3						2	6		
% Pedestrians						100.0%	50.0%						100.0%	85.7%		
Bicycles on Crosswalk						0	3						0	1		
% Bicycles on Crosswalk						0.0%	50.0%						0.0%	14.3%		

Start Time	Main Northbound						Madison Eastbound									
	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Right	Thru	Left	U-Turn	App Total	Peds CW	Peds CCW	Int Total	
2023-02-22 06:00:00	1	7	1	0	9	0	0	2	0	0	0	2	0	0	0	29
2023-02-22 06:15:00	1	9	2	0	12	0	0	3	0	0	0	3	0	0	0	37
2023-02-22 06:30:00	0	12	4	0	16	0	0	2	1	0	0	3	0	0	0	47
2023-02-22 06:45:00	0	26	1	0	27	0	0	1	0	1	0	2	0	0	0	54
2023-02-22 07:00:00	2	14	0	0	16	0	0	1	0	1	0	2	0	0	0	64
2023-02-22 07:15:00	2	21	0	0	23	0	0	4	1	0	0	5	0	0	0	83
2023-02-22 07:30:00	2	27	6	0	35	0	0	15	2	1	0	18	0	1	0	168
2023-02-22 07:45:00	7	70	7	0	84	0	0	11	2	0	0	13	0	0	0	184
2023-02-22 08:00:00	3	41	7	0	51	0	0	3	2	1	0	6	0	0	0	106
2023-02-22 08:15:00	1	28	3	0	32	0	0	1	2	0	0	3	0	0	0	73
2023-02-22 08:30:00	2	28	4	0	34	0	0	0	2	0	0	2	0	0	0	77
2023-02-22 08:45:00	1	27	2	0	30	0	0	4	4	1	0	9	0	0	0	75
2023-02-22 09:00:00	3	20	0	0	23	0	0	2	1	4	0	7	0	0	0	64
2023-02-22 09:15:00	4	30	3	0	37	0	0	4	3	5	0	12	0	0	0	94
2023-02-22 09:30:00	1	22	0	0	23	0	0	1	4	2	0	7	0	0	0	64
2023-02-22 09:45:00	3	37	2	0	42	0	1	2	7	2	0	11	0	0	0	99
2023-02-22 10:00:00	5	28	3	0	36	0	0	1	2	1	0	4	0	0	0	72
2023-02-22 10:15:00	4	16	2	0	22	0	0	3	5	2	0	10	0	0	0	79
2023-02-22 10:30:00	5	24	1	0	30	0	0	6	2	4	0	12	0	0	0	80
2023-02-22 10:45:00	5	28	5	0	38	0	0	0	4	0	0	4	0	0	0	71
2023-02-22 11:00:00	3	39	2	0	44	0	0	6	2	2	0	10	0	0	0	82
2023-02-22 11:15:00	4	27	4	0	35	0	0	5	5	3	0	13	0	0	0	90
2023-02-22 11:30:00	5	40	2	0	47	0	1	6	9	2	0	17	0	0	0	105
2023-02-22 11:45:00	6	35	5	0	46	0	0	4	3	1	0	8	0	0	0	97
2023-02-22 12:00:00	3	34	4	0	41	0	0	7	4	2	0	13	0	0	0	96
2023-02-22 12:15:00	3	31	2	0	36	0	0	2	2	2	0	6	0	0	0	82
2023-02-22 12:30:00	2	27	5	0	34	1	1	6	3	6	0	15	1	1	1	96
2023-02-22 12:45:00	3	26	3	0	32	0	0	2	3	0	0	5	0	0	0	88
2023-02-22 13:00:00	2	32	6	0	40	0	0	5	7	4	0	16	0	0	0	92
2023-02-22 13:15:00	4	25	5	0	34	0	0	2	8	2	0	12	0	0	0	82
2023-02-22 13:30:00	6	18	2	0	26	0	0	9	4	1	0	14	0	0	0	80
2023-02-22 13:45:00	1	19	3	0	23	0	0	3	2	2	0	7	0	0	0	71
2023-02-22 14:00:00	0	39	5	0	44	0	0	6	1	1	0	8	0	0	0	80
2023-02-22 14:15:00	4	26	1	0	31	0	0	3	2	1	0	6	0	0	0	77
2023-02-22 14:30:00	2	16	2	0	20	0	0	2	3	3	0	8	0	0	0	81
2023-02-22 14:45:00	1	23	2	0	26	0	0	3	3	5	0	11	0	0	0	91
2023-02-22 15:00:00	3	82	17	0	102	0	0	6	4	4	0	14	0	0	0	145
2023-02-22 15:15:00	5	43	6	0	54	1	0	5	6	7	0	18	0	0	0	134
2023-02-22 15:30:00	2	49	1	0	52	0	0	5	0	4	0	9	0	0	0	110
2023-02-22 15:45:00	5	48	5	0	58	0	0	8	2	1	0	11	0	0	0	109
2023-02-22 16:00:00	1	40	11	0	52	0	0	6	6	2	0	14	0	0	0	121
2023-02-22 16:15:00	3	48	10	0	61	0	0	6	5	2	0	13	1	0	0	114
2023-02-22 16:30:00	5	43	4	0	52	0	0	11	4	3	0	18	0	0	0	126
2023-02-22 16:45:00	4	37	2	0	43	0	1	5	1	3	0	9	0	0	0	95
2023-02-22 17:00:00	4	62	7	0	73	0	0	3	5	5	0	13	0	0	0	134
2023-02-22 17:15:00	4	60	7	0	71	0	2	7	7	7	0	21	0	0	0	140
2023-02-22 17:30:00	4	44	5	0	53	1	0	8	8	4	0	20	2	0	0	128
2023-02-22 17:45:00	4	47	6	0	57	0	0	8	10	5	0	23	0	0	0	145
2023-02-22 18:00:00	3	36	4	0	43	0	0	3	3	1	0	7	0	0	0	87
2023-02-22 18:15:00	2	23	4	0	29	0	0	7	1	1	0	9	0	0	0	63
2023-02-22 18:30:00	0	19	1	0	20	0	0	2	1	4	0	7	0	0	0	48
2023-02-22 18:45:00	1	32	6	0	39	0	0	1	2	0	0	3	0	0	0	71
Grand Total	151	1685	202	0	2038	3	6	228	170	115	0	513	4	2	2	4780
% Approach	7.4%	82.7%	9.9%	0.0%				44.4%	33.1%	22.4%	0.0%					
% Total	3.2%	35.3%	4.2%	0.0%	42.6%			4.8%	3.6%	2.4%	0.0%	10.7%				
Lights	147	1626	195	0	1968			222	169	109	0	500				4624
% Lights	97.4%	96.5%	96.5%	0.0%	96.6%			97.4%	99.4%	94.8%	0.0%	97.5%				96.7%
Articulated Trucks	0	19	0	0	19			1	0	1	0	2				42
% Articulated Trucks	0.0%	1.1%	0.0%	0.0%	0.9%			0.4%	0.0%	0.9%	0.0%	0.4%				0.9%
Buses and Single-Urn	4	40	7	0	51			5	1	5	0	11				114
% Buses and Single-	2.6%	2.4%	3.5%	0.0%	2.5%			2.2%	0.6%	4.3%	0.0%	2.1%				2.4%
Pedestrians						3	6						4	2		
% Pedestrians						#####	#####						#####	#####		
Bicycles on Crosswalk						0	0						0	0		
% Bicycles on Crosswalk						0.0%	0.0%						0.0%	0.0%		

APPENDIX B

SYNCHRO RESULTS SIGNALIZED

Lanes, Volumes, Timings
4: West St & Jefferson St (Old 28)

02/28/2023

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Volume (vph)	3	244	25	11	244	8	23	7	16	5	16	3
Future Volume (vph)	3	244	25	11	244	8	23	7	16	5	16	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	16	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	50		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	1770	1837	0	1770	1853	0	0	1962	0	0	1813	0
Flt Permitted	0.577			0.547				0.817			0.909	
Satd. Flow (perm)	1075	1837	0	1019	1853	0	0	1642	0	0	1664	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			5			24			6	
Link Speed (mph)		30			25			30			30	
Link Distance (ft)		1020			478			486			753	
Travel Time (s)		23.2			13.0			11.0			17.1	
Lane Group Flow (vph)	4	358	0	13	300	0	0	68	0	0	48	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Total Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Lost Time (s)	5.5	5.5		5.5	5.5			5.8			5.8	
Act Effct Green (s)	48.2	48.2		48.2	48.2			7.2			7.3	
Actuated g/C Ratio	0.80	0.80		0.80	0.80			0.12			0.12	
v/c Ratio	0.00	0.24		0.02	0.20			0.31			0.23	
Control Delay	3.7	3.6		2.5	2.3			20.6			23.7	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.7	3.6		2.5	2.3			20.6			23.7	
LOS	A	A		A	A			C			C	
Approach Delay		3.6			2.3			20.6			23.7	
Approach LOS		A			A			C			C	
Stops (vph)	2	73		3	41			30			20	
Fuel Used(gal)	0	3		0	1			1			0	
CO Emissions (g/hr)	3	191		4	91			37			26	
NOx Emissions (g/hr)	1	37		1	18			7			5	
VOC Emissions (g/hr)	1	44		1	21			9			6	
Dilemma Vehicles (#)	0	0		0	0			0			0	
Queue Length 50th (ft)	1	36		1	21			15			14	
Queue Length 95th (ft)	2	60		m4	38			31			20	
Internal Link Dist (ft)		940			398			406			673	
Turn Bay Length (ft)		50			150							
Base Capacity (vph)	864	1479		819	1491			406			398	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.00	0.24		0.02	0.20			0.17			0.12	
Intersection Summary												

Lanes, Volumes, Timings

4: West St & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 47 (78%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.31

Intersection Signal Delay: 5.7

Intersection LOS: A

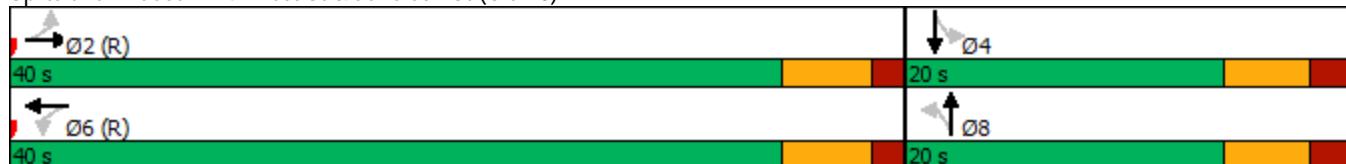
Intersection Capacity Utilization 29.3%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: West St & Jefferson St (Old 28)



Lanes, Volumes, Timings

7: Main St (Old 19) & Jefferson St (Old 28)

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓		↑	↓		↑	↓	
Traffic Volume (vph)	14	185	66	97	214	18	32	58	66	16	117	21
Future Volume (vph)	14	185	66	97	214	18	32	58	66	16	117	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	150		0	190		0	30		0	50		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			100			50			100		
Satd. Flow (prot)	1770	1788	0	1770	1840	0	1770	1714	0	1770	1820	0
Flt Permitted	0.593			0.563			0.634			0.610		
Satd. Flow (perm)	1105	1788	0	1049	1840	0	1181	1714	0	1136	1820	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)	36			8			112			18		
Link Speed (mph)	25			25			30			30		
Link Distance (ft)	478			471			476			769		
Travel Time (s)	13.0			12.8			10.8			17.5		
Lane Group Flow (vph)	18	326	0	113	270	0	54	210	0	23	197	0
Turn Type	Perm	NA										
Protected Phases	2			6			8			4		
Permitted Phases	2		6			8			4			
Total Split (s)	30.0	30.0		30.0	30.0		30.0	30.0		30.0	30.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0		6.0	6.0		6.0	6.0	
Act Effct Green (s)	35.9	35.9		35.9	35.9		12.1	12.1		12.1	12.1	
Actuated g/C Ratio	0.60	0.60		0.60	0.60		0.20	0.20		0.20	0.20	
v/c Ratio	0.03	0.30		0.18	0.24		0.23	0.48		0.10	0.52	
Control Delay	3.6	4.4		4.2	4.2		17.8	10.8		19.2	24.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	3.6	4.4		4.2	4.2		17.8	10.8		19.2	24.0	
LOS	A	A		A	A		B	B		B	C	
Approach Delay		4.3			4.2			12.2			23.5	
Approach LOS		A			A			B			C	
Stops (vph)	3	82		24	64		25	71		15	106	
Fuel Used(gal)	0	2		1	1		0	1		0	2	
CO Emissions (g/hr)	6	109		39	96		26	79		17	146	
NOx Emissions (g/hr)	1	21		8	19		5	15		3	28	
VOC Emissions (g/hr)	1	25		9	22		6	18		4	34	
Dilemma Vehicles (#)	0	0		0	0		0	0		0	0	
Queue Length 50th (ft)	1	20		7	16		17	32		7	59	
Queue Length 95th (ft)	4	28		15	28		14	10		17	75	
Internal Link Dist (ft)		398			391			396			689	
Turn Bay Length (ft)	150		190			30			50			
Base Capacity (vph)	661	1085		628	1105		472	752		454	738	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.03	0.30		0.18	0.24		0.11	0.28		0.05	0.27	

Intersection Summary

Lanes, Volumes, Timings

7: Main St (Old 19) & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 4 (7%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.52

Intersection Signal Delay: 9.5

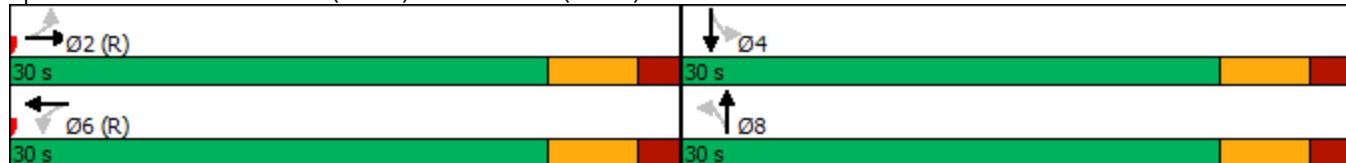
Intersection LOS: A

Intersection Capacity Utilization 58.8%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Main St (Old 19) & Jefferson St (Old 28)



Lanes, Volumes, Timings
10: Main St (Old 19) & Madison St

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	2	7	33	13	4	7	20	159	14	5	264	13
Future Volume (vph)	2	7	33	13	4	7	20	159	14	5	264	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	0	1660	0	0	1742	0	0	1835	0	0	1850	0
Flt Permitted		0.983			0.790			0.937			0.994	
Satd. Flow (perm)	0	1635	0	0	1414	0	0	1728	0	0	1840	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)	57			9			11			7		
Link Speed (mph)	30			30			30			30		
Link Distance (ft)	483			731			727			476		
Travel Time (s)	11.0			16.6			16.5			10.8		
Lane Group Flow (vph)	0	72	0	0	31	0	0	339	0	0	428	0
Turn Type	Perm	NA										
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Total Split (s)	20.0	20.0	20.0	20.0		40.0	40.0		40.0	40.0		
Total Lost Time (s)	6.0			6.0			6.0			6.0		
Act Effct Green (s)	6.7			6.7			48.3			48.3		
Actuated g/C Ratio	0.11			0.11			0.80			0.80		
v/c Ratio	0.31			0.19			0.24			0.29		
Control Delay	13.8			21.3			3.6			3.8		
Queue Delay	0.0			0.0			0.0			0.0		
Total Delay	13.8			21.3			3.6			3.8		
LOS	B		C			A			A			
Approach Delay	13.8			21.3			3.6			3.8		
Approach LOS	B		C			A			A			
Stops (vph)	17			19			54			85		
Fuel Used(gal)	0		0			2			2			
CO Emissions (g/hr)	26			23			107			121		
NOx Emissions (g/hr)	5			5			21			24		
VOC Emissions (g/hr)	6			5			25			28		
Dilemma Vehicles (#)	0			0			0			0		
Queue Length 50th (ft)	5			7			35			57		
Queue Length 95th (ft)	15			22			40			73		
Internal Link Dist (ft)	403			651			647			396		
Turn Bay Length (ft)												
Base Capacity (vph)	425			336			1392			1482		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.17			0.09			0.24			0.29		

Intersection Summary

Lanes, Volumes, Timings

10: Main St (Old 19) & Madison St

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 23 (38%), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.31

Intersection Signal Delay: 5.2

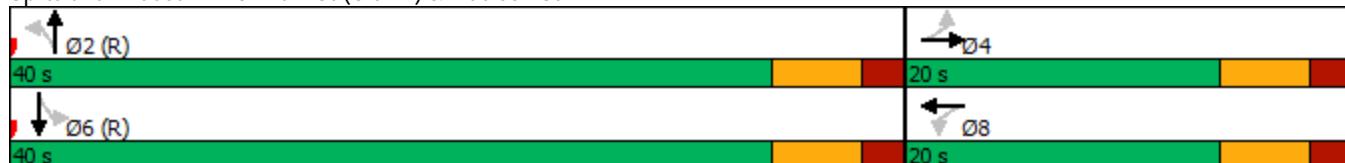
Intersection LOS: A

Intersection Capacity Utilization 37.5%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 10: Main St (Old 19) & Madison St



Lanes, Volumes, Timings

13: Independence St & Jefferson St (Old 28)

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Volume (vph)	5	239	6	17	325	14	6	8	13	9	14	16
Future Volume (vph)	5	239	6	17	325	14	6	8	13	9	14	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	100		0	75		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	1770	1857	0	1770	1852	0	0	1724	0	0	1741	0
Flt Permitted	0.546			0.576				0.906			0.908	
Satd. Flow (perm)	1017	1857	0	1073	1852	0	0	1580	0	0	1598	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3			5			21			21	
Link Speed (mph)		25			25			30			30	
Link Distance (ft)		471			605			1019			765	
Travel Time (s)		12.8			16.5			23.2			17.4	
Lane Group Flow (vph)	6	302	0	18	361	0	0	44	0	0	52	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Total Split (s)	35.0	35.0		35.0	35.0		25.0	25.0		25.0	25.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0			6.0			6.0	
Act Effct Green (s)	48.2	48.2		48.2	48.2			6.8			6.8	
Actuated g/C Ratio	0.80	0.80		0.80	0.80			0.11			0.11	
v/c Ratio	0.01	0.20		0.02	0.24			0.22			0.26	
Control Delay	2.6	2.3		3.6	3.7			18.2			19.6	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	2.6	2.3		3.6	3.7			18.2			19.6	
LOS	A	A		A	A			B			B	
Approach Delay		2.3			3.7			18.2			19.6	
Approach LOS		A			A			B			B	
Stops (vph)	2	41		7	95			17			25	
Fuel Used(gal)	0	1		0	2			0			1	
CO Emissions (g/hr)	2	88		9	165			29			37	
NOx Emissions (g/hr)	0	17		2	32			6			7	
VOC Emissions (g/hr)	0	20		2	38			7			9	
Dilemma Vehicles (#)	0	0		0	0			0			0	
Queue Length 50th (ft)	1	22		2	38			8			10	
Queue Length 95th (ft)	m2	36		7	80			19			29	
Internal Link Dist (ft)		391			525			939			685	
Turn Bay Length (ft)	100			75								
Base Capacity (vph)	816	1492		861	1488			514			520	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.01	0.20		0.02	0.24			0.09			0.10	

Intersection Summary

Lanes, Volumes, Timings

13: Independence St & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 52 (87%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.26

Intersection Signal Delay: 5.0

Intersection LOS: A

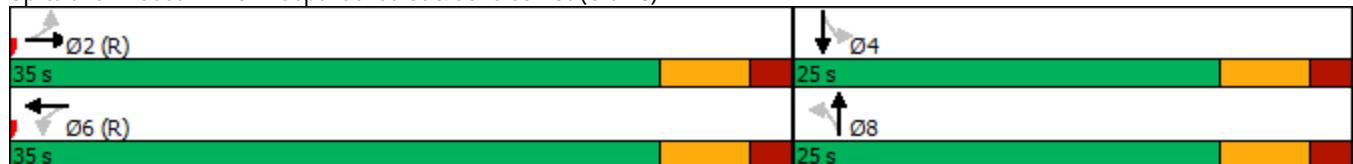
Intersection Capacity Utilization 32.1%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Independence St & Jefferson St (Old 28)



Lanes, Volumes, Timings
4: West St & Jefferson St (Old 28)

02/28/2023

	→	→	→	←	←	←	↑	↑	↓	↓	↑	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑			↔			↔	
Traffic Volume (vph)	7	324	17	38	277	20	29	9	33	8	14	3
Future Volume (vph)	7	324	17	38	277	20	29	9	33	8	14	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	16	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	50		0	150		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	1770	1848	0	1770	1844	0	0	1939	0	0	1805	0
Flt Permitted	0.560			0.548				0.851			0.863	
Satd. Flow (perm)	1043	1848	0	1021	1844	0	0	1683	0	0	1583	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8			10			41			4	
Link Speed (mph)		30			25			30			30	
Link Distance (ft)		1020			478			486			753	
Travel Time (s)		23.2			13.0			11.0			17.1	
Lane Group Flow (vph)	7	356	0	43	333	0	0	88	0	0	36	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Total Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Lost Time (s)	5.5	5.5		5.5	5.5			5.8			5.8	
Act Effct Green (s)	44.7	44.7		44.7	44.7			7.4			7.4	
Actuated g/C Ratio	0.74	0.74		0.74	0.74			0.12			0.12	
v/c Ratio	0.01	0.26		0.06	0.24			0.36			0.18	
Control Delay	3.6	4.2		2.7	2.7			18.7			22.9	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	3.6	4.2		2.7	2.7			18.7			22.9	
LOS	A	A		A	A			B			C	
Approach Delay		4.2			2.7			18.7			22.9	
Approach LOS		A			A			B			C	
Stops (vph)	3	110		10	57			39			21	
Fuel Used(gal)	0	4		0	2			1			0	
CO Emissions (g/hr)	5	253		15	111			52			27	
NOx Emissions (g/hr)	1	49		3	22			10			5	
VOC Emissions (g/hr)	1	59		3	26			12			6	
Dilemma Vehicles (#)	0	0		0	0			0			0	
Queue Length 50th (ft)	1	37		3	22			16			11	
Queue Length 95th (ft)	4	80		9	45			41			23	
Internal Link Dist (ft)		940			398			406			673	
Turn Bay Length (ft)	50			150								
Base Capacity (vph)	776	1377		760	1375			429			377	
Starvation Cap Reductn	0	0		0	0			0			0	
Spillback Cap Reductn	0	0		0	0			0			0	
Storage Cap Reductn	0	0		0	0			0			0	
Reduced v/c Ratio	0.01	0.26		0.06	0.24			0.21			0.10	

Intersection Summary

Lanes, Volumes, Timings

4: West St & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 55 (92%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.36

Intersection Signal Delay: 5.8

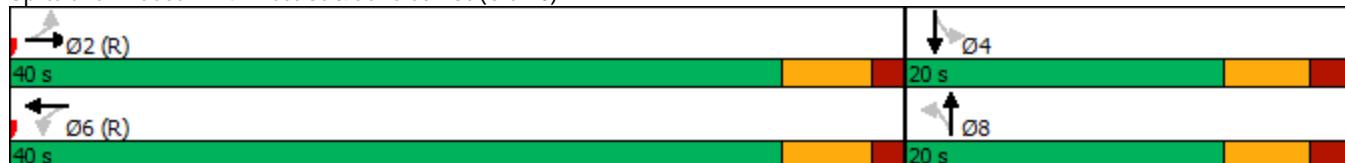
Intersection LOS: A

Intersection Capacity Utilization 46.4%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 4: West St & Jefferson St (Old 28)



Lanes, Volumes, Timings

7: Main St (Old 19) & Jefferson St (Old 28)

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓		↑	↓		↑	↓	
Traffic Volume (vph)	26	323	38	66	259	32	58	70	96	22	71	19
Future Volume (vph)	26	323	38	66	259	32	58	70	96	22	71	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	150		0	190		0	30		0	50		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			100			50			100		
Satd. Flow (prot)	1770	1833	0	1770	1831	0	1770	1703	0	1770	1803	0
Flt Permitted	0.563			0.537			0.673			0.625		
Satd. Flow (perm)	1049	1833	0	1000	1831	0	1254	1703	0	1164	1803	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			14			114			24	
Link Speed (mph)		25			25			30			30	
Link Distance (ft)		478			471			476			769	
Travel Time (s)		13.0			12.8			10.8			17.5	
Lane Group Flow (vph)	27	376	0	74	327	0	72	200	0	32	131	0
Turn Type	Perm	NA										
Protected Phases		2			6			8			4	
Permitted Phases	2			6			8			4		
Total Split (s)	35.0	35.0		35.0	35.0		25.0	25.0		25.0	25.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0		6.0	6.0		6.0	6.0	
Act Effct Green (s)	37.0	37.0		37.0	37.0		11.0	11.0		11.0	11.0	
Actuated g/C Ratio	0.62	0.62		0.62	0.62		0.18	0.18		0.18	0.18	
v/c Ratio	0.04	0.33		0.12	0.29		0.31	0.49		0.15	0.37	
Control Delay	3.4	4.4		3.4	4.0		21.2	11.7		21.5	20.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	3.4	4.4		3.4	4.0		21.2	11.7		21.5	20.5	
LOS	A	A		A	A		C	B		C	C	
Approach Delay		4.3			3.9			14.2			20.7	
Approach LOS		A			A			B			C	
Stops (vph)	6	108		13	74		51	101		20	62	
Fuel Used(gal)	0	2		0	2		1	2		0	1	
CO Emissions (g/hr)	10	154		25	118		52	109		24	88	
NOx Emissions (g/hr)	2	30		5	23		10	21		5	17	
VOC Emissions (g/hr)	2	36		6	27		12	25		5	20	
Dilemma Vehicles (#)	0	0		0	0		0	0		0	0	
Queue Length 50th (ft)	2	27		5	18		24	29		10	35	
Queue Length 95th (ft)	7	47		12	34		47	64		21	51	
Internal Link Dist (ft)		398			391			396			689	
Turn Bay Length (ft)	150			190			30			50		
Base Capacity (vph)	646	1134		616	1133		397	617		368	587	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.04	0.33		0.12	0.29		0.18	0.32		0.09	0.22	

Intersection Summary

Lanes, Volumes, Timings

7: Main St (Old 19) & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 8 (13%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 8.5

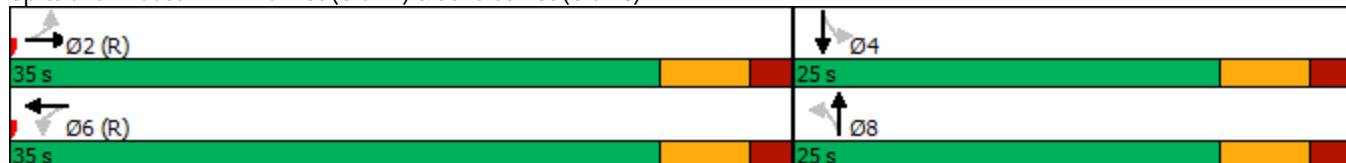
Intersection LOS: A

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 7: Main St (Old 19) & Jefferson St (Old 28)



Lanes, Volumes, Timings
10: Main St (Old 19) & Madison St

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	18	17	26	12	4	9	20	202	17	14	137	19
Future Volume (vph)	18	17	26	12	4	9	20	202	17	14	137	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	0	1728	0	0	1731	0	0	1837	0	0	1827	0
Flt Permitted		0.886			0.803			0.972			0.971	
Satd. Flow (perm)	0	1555	0	0	1424	0	0	1792	0	0	1782	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)	36			14			10			17		
Link Speed (mph)	30			30			30			30		
Link Distance (ft)	483			731			727			476		
Travel Time (s)	11.0			16.6			16.5			10.8		
Lane Group Flow (vph)	0	84	0	0	39	0	0	291	0	0	185	0
Turn Type	Perm	NA										
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Total Split (s)	21.0	21.0	21.0	21.0		39.0	39.0		39.0	39.0		
Total Lost Time (s)	6.0			6.0			6.0			6.0		
Act Effct Green (s)	7.6			7.6			43.9			43.9		
Actuated g/C Ratio	0.13			0.13			0.73			0.73		
v/c Ratio	0.37			0.20			0.22			0.14		
Control Delay	19.8			19.0			4.3			3.1		
Queue Delay	0.0			0.0			0.0			0.0		
Total Delay	19.8			19.0			4.3			3.1		
LOS	B			B			A			A		
Approach Delay	19.8			19.0			4.3			3.1		
Approach LOS	B			B			A			A		
Stops (vph)	36			17			79			54		
Fuel Used(gal)	1			0			2			1		
CO Emissions (g/hr)	47			23			140			72		
NOx Emissions (g/hr)	9			5			27			14		
VOC Emissions (g/hr)	11			5			32			17		
Dilemma Vehicles (#)	0			0			0			0		
Queue Length 50th (ft)	16			8			31			21		
Queue Length 95th (ft)	36			19			61			38		
Internal Link Dist (ft)	403			651			647			396		
Turn Bay Length (ft)												
Base Capacity (vph)	415			366			1315			1309		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	0.20			0.11			0.22			0.14		

Intersection Summary

Lanes, Volumes, Timings

10: Main St (Old 19) & Madison St

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 28 (47%), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.37

Intersection Signal Delay: 7.1

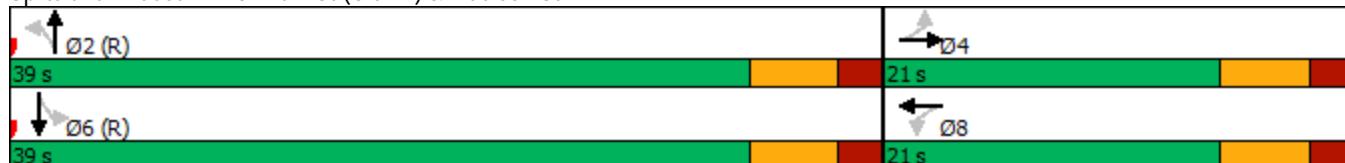
Intersection LOS: A

Intersection Capacity Utilization 31.0%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 10: Main St (Old 19) & Madison St



Lanes, Volumes, Timings

13: Independence St & Jefferson St (Old 28)

02/28/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Volume (vph)	17	401	14	15	336	24	13	10	32	10	14	24
Future Volume (vph)	17	401	14	15	336	24	13	10	32	10	14	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	100		0	75		0	0		0	0		0
Storage Lanes	1		0	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Satd. Flow (prot)	1770	1853	0	1770	1844	0	0	1695	0	0	1719	0
Flt Permitted	0.533			0.490				0.901			0.913	
Satd. Flow (perm)	993	1853	0	913	1844	0	0	1546	0	0	1585	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)	5			10			35			32		
Link Speed (mph)	25			25			30			30		
Link Distance (ft)	471			605			1019			765		
Travel Time (s)	12.8			16.5			23.2			17.4		
Lane Group Flow (vph)	20	477	0	16	387	0	0	60	0	0	64	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	2			6			8			4		
Permitted Phases	2		6		8			4				
Total Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Lost Time (s)	6.0	6.0		6.0	6.0		6.0			6.0		
Act Effct Green (s)	48.0	48.0		48.0	48.0		7.0			7.0		
Actuated g/C Ratio	0.80	0.80		0.80	0.80		0.12			0.12		
v/c Ratio	0.03	0.32		0.02	0.26		0.29			0.30		
Control Delay	2.7	2.8		3.7	3.8		16.8			18.1		
Queue Delay	0.0	0.0		0.0	0.0		0.0			0.0		
Total Delay	2.7	2.8		3.7	3.8		16.8			18.1		
LOS	A	A		A	A		B			B		
Approach Delay		2.8			3.8		16.8			18.1		
Approach LOS		A			A		B			B		
Stops (vph)	5	77		7	102		29			27		
Fuel Used(gal)	0	2		0	3		1			1		
CO Emissions (g/hr)	7	154		8	176		55			43		
NOx Emissions (g/hr)	1	30		2	34		11			8		
VOC Emissions (g/hr)	2	36		2	41		13			10		
Dilemma Vehicles (#)	0	0		0	0		0			0		
Queue Length 50th (ft)	2	41		1	42		8			11		
Queue Length 95th (ft)	m5	66		7	87		36			31		
Internal Link Dist (ft)		391			525		939			685		
Turn Bay Length (ft)	100		75									
Base Capacity (vph)	795	1484		731	1478		387			394		
Starvation Cap Reductn	0	0		0	0		0			0		
Spillback Cap Reductn	0	0		0	0		0			0		
Storage Cap Reductn	0	0		0	0		0			0		
Reduced v/c Ratio	0.03	0.32		0.02	0.26		0.16			0.16		

Intersection Summary

Lanes, Volumes, Timings

13: Independence St & Jefferson St (Old 28)

02/28/2023

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 60

Offset: 55 (92%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.32

Intersection Signal Delay: 5.0

Intersection LOS: A

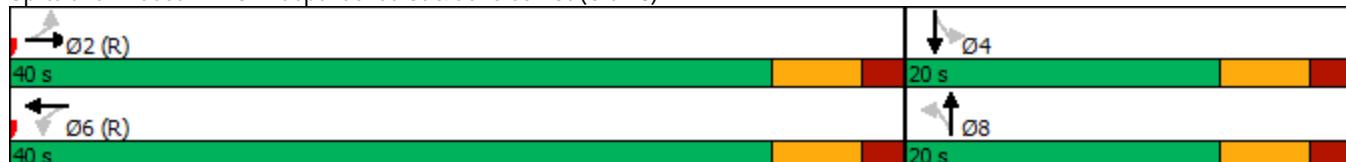
Intersection Capacity Utilization 36.4%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 13: Independence St & Jefferson St (Old 28)



APPENDIX C

SIGNAL WARRANT RESULTS

Summary of Warrants

Spot Number:	0				
Major Street:	Jefferson St	Minor Street:	West St		
Intersection:	Jefferson St at West St				
City/Twp:	Tipton, IN				
Date Performed:	2/28/2023	Performed By:	WBS		
Date Volumes Collected:	2/22/2023				
Warrant	Condition	Is Warrant Met			
Data Validation Error		NO			
WARRANT 1: Eight-Hour Vehicular Volume		NO			
	Condition A	NO			
	Condition B	NO			
	Condition A&B	N/A			
WARRANT 2: Four-Hour Vehicular Volume	(70%)	NO			
WARRANT 3: Peak-Hour Vehicular Volume	(70%)	#N/A			
	Condition A	#N/A			
	Condition B	NO			
WARRANT 4: Pedestrian Volume	(70%)	NO			
	Four Hour	N/A			
	Peak Hour	N/A			
	(Threshold)	HAWK	NO		
	(Threshold)	RRFB	NO		
WARRANT 5: School Crossing		NO			
WARRANT 6: Coordinated Signal System		NO			
WARRANT 7: Crash Experience		NO			
	Condition A	NO			
	Condition B	NO			
WARRANT 8: Roadway Network		NO			
WARRANT 9: Intersection Near a Grade Crossing		#N/A			
Issue to Be Addressed by Signalization:					
0					

Indiana Manual of Uniform Traffic Control Devices
Worksheet for Signal Warrants (Section 4C)
WARRANT 1: Eight-Hour Vehicular Volume

Intersection:	Jefferson St @ West St	
Date	2/28/2023	by WBS

2	: No. of Lanes on Major St?
1	: No. of Lanes on Minor St?
0	: Speed limit or 85th Percentile? (MPH)
YES	: Is the intersection within an Isolated community?
5275	: if answer 4 is Yes, then what is the of the population isolated community?
NO	: Have other remedial measures been tried?

USE 70% WARRANTS 1A AND 1B. DO NOT USE COMBINATION OF A & B

	Major Volume (Both Apr.)	Minor Volume (One Apr.)	Condition A Major Volume	Condition A Minor Volume	Warrant Condition A Met?	Condition B Major Volume	Condition B Minor Volume	Warrant Condition B Met?	Combination Major A	Combination Minor A	Combination Major B	Combination Minor B	Warrant Condition A&B met?
Time	E-W	N-S											
00:01 - 01:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
01:00 - 02:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
02:00 - 03:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
03:00 - 04:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
04:00 - 05:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
05:00 - 06:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
06:00 - 07:00	401	26	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
07:00 - 08:00	526	43	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
08:00 - 09:00	395	33	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
09:00 - 10:00	422	37	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
10:00 - 11:00	375	45	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
11:00 - 12:00	491	56	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
12:00 - 13:00	463	64	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
13:00 - 14:00	391	52	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
14:00 - 15:00	452	46	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
15:00 - 16:00	633	58	420	105	NO	630	53	YES	N/A	N/A	N/A	N/A	N/A
16:00 - 17:00	634	58	420	105	NO	630	53	YES	N/A	N/A	N/A	N/A	N/A
17:00 - 18:00	653	74	420	105	NO	630	53	YES	N/A	N/A	N/A	N/A	N/A
18:00 - 19:00	382	46	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
19:00 - 20:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
20:00 - 21:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
21:00 - 22:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
22:00 - 23:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
23:00 - 00:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A

Number of Hours that met the warrant 1A =
Number of Hours that met the warrant 1B =
Number of Hours that met the warrant 1 A & B =

A. Is the Minimum Vehicular Volume Warrant Met? (Condition A)	NO
B. Is the Interruption of Continuous Traffic Met? (Condition B)	NO
C. Combination of Warrants A and B Criteria Met?	N/A

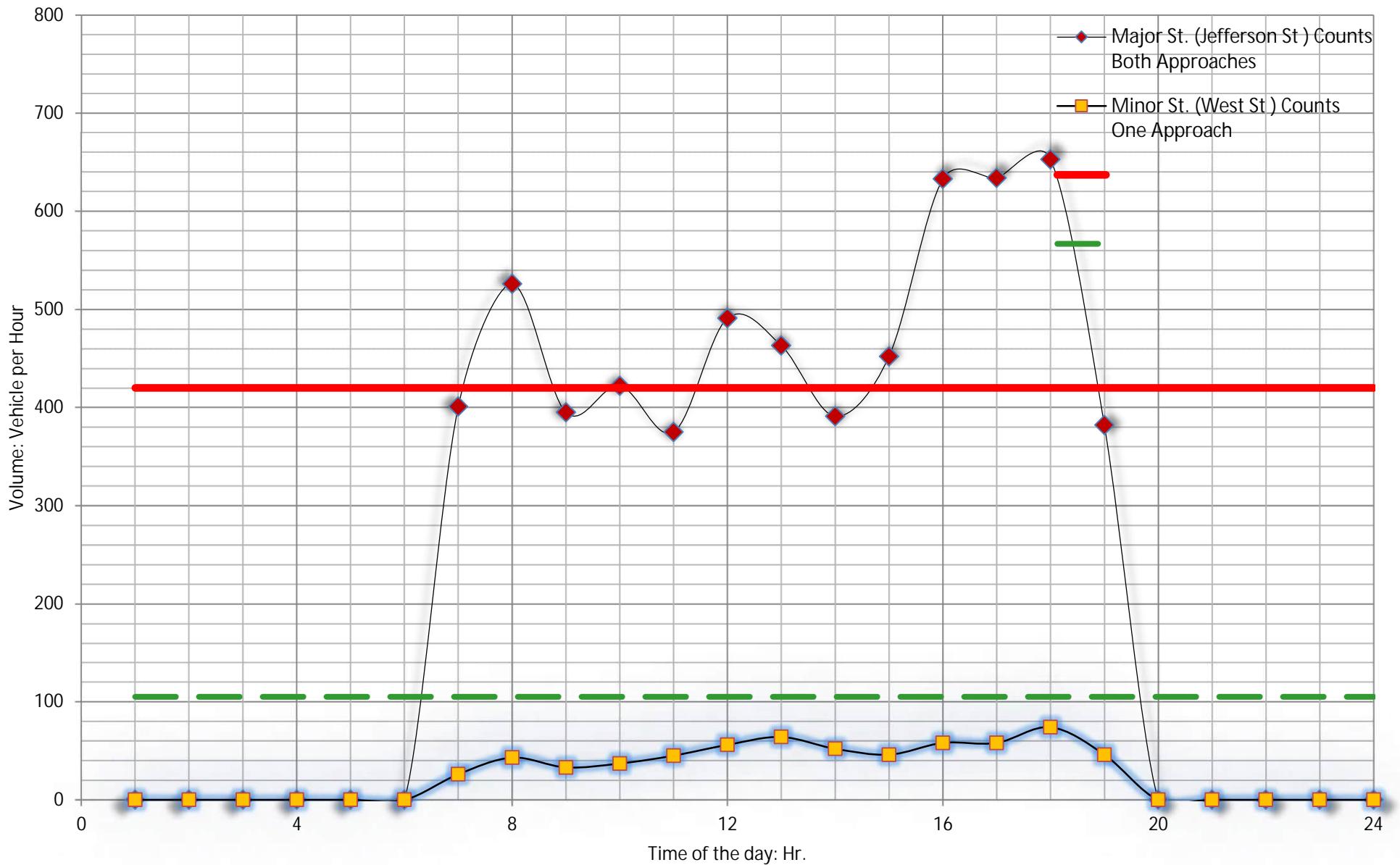


FIGURE 1: WARRANT 1A

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? **NO**

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? **YES**

Spot Number:

Jefferson St @ West St

NO. OF LANES ON MAJOR ST.? **2**

NO. OF LANES ON MINOR ST.? **1**

Number of Hours that met the Warrant: **0**

Does this intersection meet Warrant 1A for signal installation? **NO**

Data Collection Date: **2/22/2023**

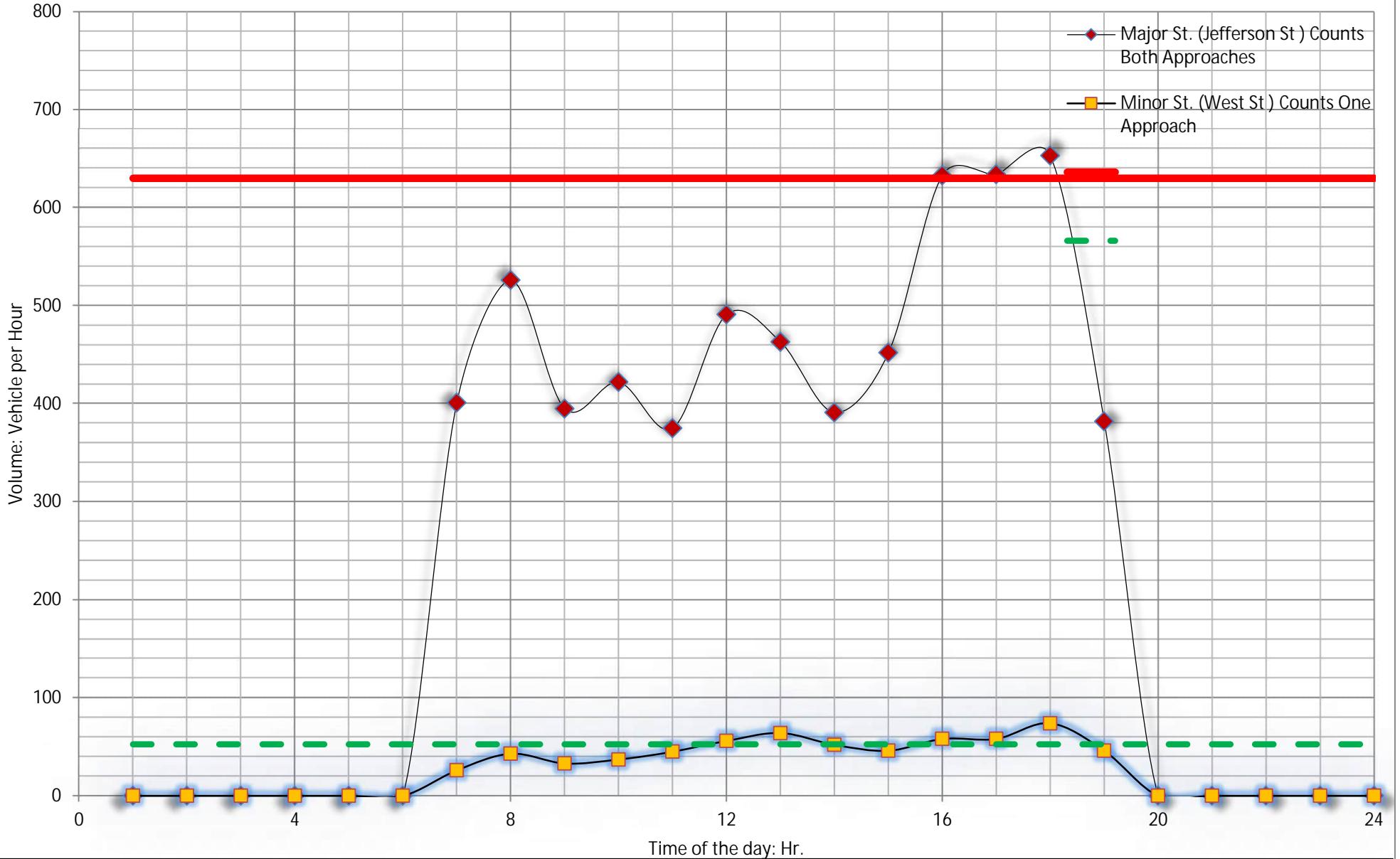


FIGURE 1: WARRANT 1B

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? NO

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? YES

Spot Number:

Jefferson St @ West St

NO. OF LANES ON MAJOR ST.? **2**
NO. OF LANES ON MINOR ST.? **1**

Number of Hours that met the Warrant: **3**

Does this intersection meet Warrant 1B **NO** for signal installation?

Data Collection Date: **2/22/2023**

Indiana Manual of Uniform Traffic Control Devices												
Worksheet for Signal Warrants (Section 4C)												
WARRANT 3 B(70%): Peak-Hour Vehicular Volume												
Spot Number:	0											
Intersection:	Jefferson St @ West St											
Date	2/28/2023	by WBS										
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td style="width: 20px; text-align: center;">2</td><td>: No. of Lanes on Major St.</td></tr> <tr><td style="text-align: center;">1</td><td>: No. of Lanes on Minor St.</td></tr> <tr><td style="text-align: center;">30</td><td>: Speed limit or 85th Percentile? (MPH)</td></tr> <tr><td style="text-align: center;">YES</td><td>: Is the intersection within an Isolated community?</td></tr> <tr><td style="text-align: center;">5275</td><td>: What is the of the population isolated community?</td></tr> </table>			2	: No. of Lanes on Major St.	1	: No. of Lanes on Minor St.	30	: Speed limit or 85th Percentile? (MPH)	YES	: Is the intersection within an Isolated community?	5275	: What is the of the population isolated community?
2	: No. of Lanes on Major St.											
1	: No. of Lanes on Minor St.											
30	: Speed limit or 85th Percentile? (MPH)											
YES	: Is the intersection within an Isolated community?											
5275	: What is the of the population isolated community?											
How Many Hours Are Met		0										
Is Warrant (70%) Met?		NO										

Summary of Warrants

Spot Number:	0	
Major Street:	Jefferson St	Minor Street: Main St
Intersection:	Jefferson St at Main St	
City/Twp:	Tipton, IN	
Date Performed:	2/28/2023	Performed By: WBS
Date Volumes Collected:	2/22/2023	
Warrant	Condition	Is Warrant Met
Data Validation Error		NO
WARRANT 1: Eight-Hour Vehicular Volume		NO
	Condition A	NO
	Condition B	NO
	Condition A&B	N/A
WARRANT 2: Four-Hour Vehicular Volume	(70%)	NO
WARRANT 3: Peak-Hour Vehicular Volume	(70%)	NO
	Condition A	NO
	Condition B	NO
WARRANT 4: Pedestrian Volume	(70%)	NO
	Four Hour	N/A
	Peak Hour	N/A
	(Threshold)	HAWK
	(Threshold)	RRFB
WARRANT 5: School Crossing		NO
WARRANT 6: Coordinated Signal System		NO
WARRANT 7: Crash Experience		NO
	Condition A	NO
	Condition B	NO
WARRANT 8: Roadway Network		NO
WARRANT 9: Intersection Near a Grade Crossing		#N/A
Issue to Be Addressed by Signalization:		
0		

Indiana Manual of Uniform Traffic Control Devices
Worksheet for Signal Warrants (Section 4C)
WARRANT 1: Eight-Hour Vehicular Volume

Intersection:	Jefferson St @ Main St	
Date	2/28/2023	by WBS

2	: No. of Lanes on Major St?
2	: No. of Lanes on Minor St?
25	: Speed limit or 85th Percentile? (MPH)
YES	: Is the intersection within an Isolated community?
5275	: if answer 4 is Yes, then what is the of the population isolated community?
NO	: Have other remedial measures been tried?

USE 70% WARRANTS 1A AND 1B. DO NOT USE COMBINATION OF A & B

	Major Volume (Both Apr.)	Minor Volume (One Apr.)	Condition A Major Volume	Condition A Minor Volume	Warrant Condition A Met?	Condition B Major Volume	Condition B Minor Volume	Warrant Condition B Met?	Combination Major A	Combination Minor A	Combination Major B	Combination Minor B	Warrant Condition A&B met?
Time	E-W	N-S											
00:01 - 01:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
01:00 - 02:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
02:00 - 03:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
03:00 - 04:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
04:00 - 05:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
05:00 - 06:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
06:00 - 07:00	437	55	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
07:00 - 08:00	589	148	420	140	YES	630	70	NO	N/A	N/A	N/A	N/A	N/A
08:00 - 09:00	449	122	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
09:00 - 10:00	502	125	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
10:00 - 11:00	435	110	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
11:00 - 12:00	543	143	420	140	YES	630	70	NO	N/A	N/A	N/A	N/A	N/A
12:00 - 13:00	552	135	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
13:00 - 14:00	475	102	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
14:00 - 15:00	558	123	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
15:00 - 16:00	681	234	420	140	YES	630	70	YES	N/A	N/A	N/A	N/A	N/A
16:00 - 17:00	712	206	420	140	YES	630	70	YES	N/A	N/A	N/A	N/A	N/A
17:00 - 18:00	713	216	420	140	YES	630	70	YES	N/A	N/A	N/A	N/A	N/A
18:00 - 19:00	425	129	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
19:00 - 20:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
20:00 - 21:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
21:00 - 22:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
22:00 - 23:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A
23:00 - 00:00	0	0	420	140	NO	630	70	NO	N/A	N/A	N/A	N/A	N/A

Number of Hours that met the warrant 1A =
 Number of Hours that met the warrant 1B =
 Number of Hours that met the warrant 1 A & B =

A. Is the Minimum Vehicular Volume Warrant Met? (Condition A)	NO
B. Is the Interruption of Continuous Traffic Met? (Condition B)	NO
C. Combination of Warrants A and B Criteria Met?	N/A

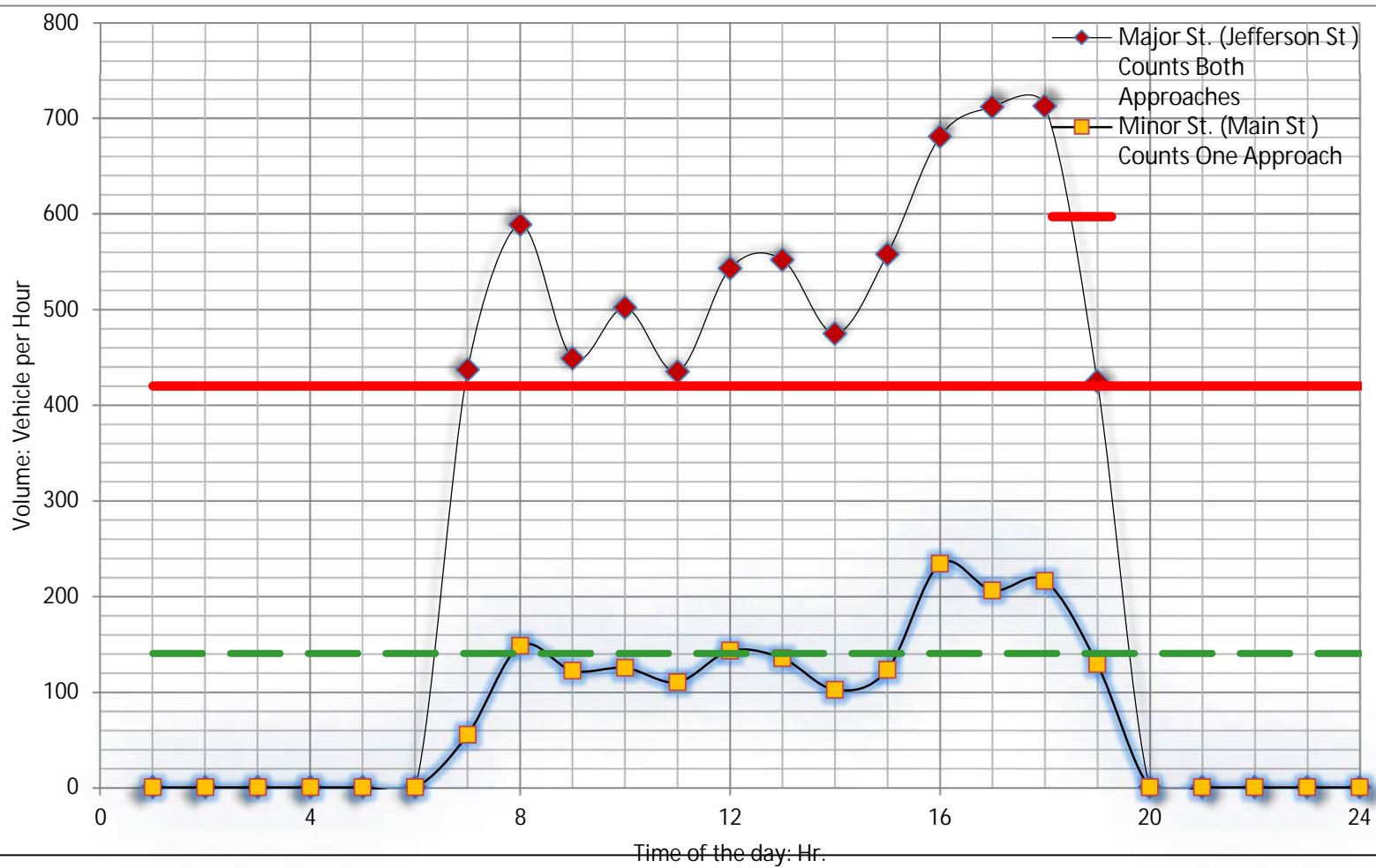


FIGURE 1: WARRANT

IS THERE A REDUCTION IN THE WARRANT
 1- DUE TO SPEED? **N**
 2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? **YES**

Spot Number:	<u>Jefferson St @ Main St</u>	Number of Hours that met the Warrant:	5
NO. OF LANES ON MAJOR STREETS ON MINOR ST.?	2	Does this intersection meet Warrant 1A for signal installation?	NO
		Data Collection Date:	2/22/2023

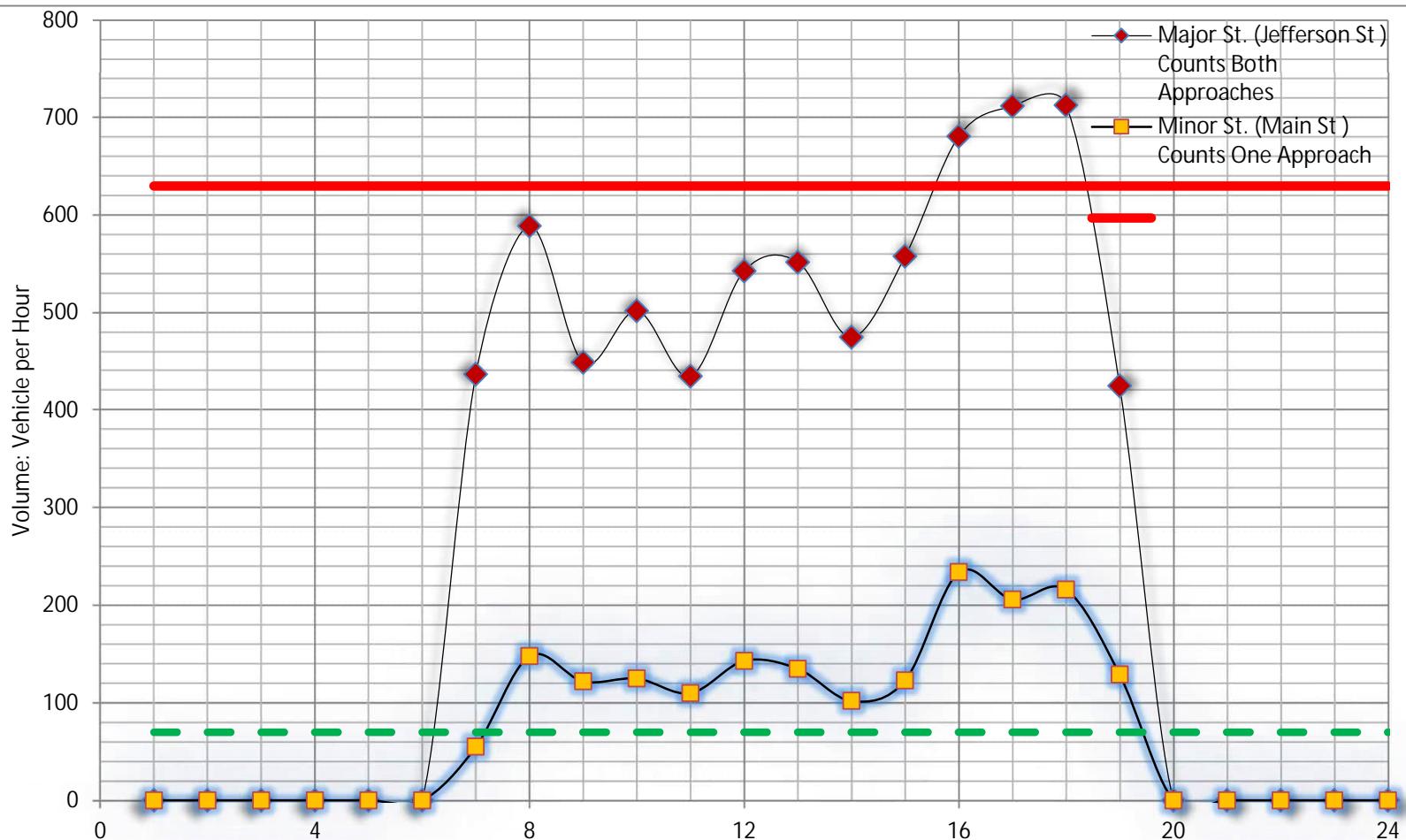


FIGURE 1: WARRANT

1B
IS THERE A REDUCTION IN THE WARRANT
THRESHOLDS TO 70% ...
1- DUE TO NO
SPEED?
2- DUE TO ISOLATED COMMUNITY WITH
POPULATION YES LESS THAN 10,000?

Spot Number:	Time of the day: Hr.	Number of Hours that met the Warrant: Does this intersection meet Warrant <u>1B</u> for signal installation?
<u>Jefferson St @ Main St</u>		<u>3</u> <u>NO</u>
NO. OF LANES ON MAJOR ST. MINOR ST.?	<u>2</u>	Data Collection Date: <u>2/22/2023</u>

Indiana Manual of Uniform Traffic Control Devices												
Worksheet for Signal Warrants (Section 4C)												
WARRANT 3 B(70%): Peak-Hour Vehicular Volume												
Spot Number:	0											
Intersection:	Jefferson St @ Main St											
Date	2/28/2023	by WBS										
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td style="width: 20px; text-align: center;">2</td><td>: No. of Lanes on Major St.</td></tr> <tr><td style="text-align: center;">2</td><td>: No. of Lanes on Minor St.</td></tr> <tr><td style="text-align: center;">25</td><td>: Speed limit or 85th Percentile? (MPH)</td></tr> <tr><td style="text-align: center;">YES</td><td>: Is the intersection within an Isolated community?</td></tr> <tr><td style="text-align: center;">5275</td><td>: What is the of the population isolated community?</td></tr> </table>			2	: No. of Lanes on Major St.	2	: No. of Lanes on Minor St.	25	: Speed limit or 85th Percentile? (MPH)	YES	: Is the intersection within an Isolated community?	5275	: What is the of the population isolated community?
2	: No. of Lanes on Major St.											
2	: No. of Lanes on Minor St.											
25	: Speed limit or 85th Percentile? (MPH)											
YES	: Is the intersection within an Isolated community?											
5275	: What is the of the population isolated community?											
<p>Scatter plot showing Minor Street Higher Volume (VPH) on the Y-axis versus Major Street - TOTAL OF BOTH APPROACHES - VEHICLES PER HOUR (VPH) on the X-axis. The X-axis ranges from 300 to 1300, and the Y-axis ranges from 0 to 500. Three reference lines are plotted: '2 or More Lanes & 2 or More Lanes' (steepest), '2 or More Lanes & 1 Lane' (middle), and '1 Lane & 1 Lane' (shallowest). Data points are represented by diamonds, showing a general upward trend as major street volume increases.</p>												
How Many Hours Are Met		0										
Is Warrant (70%) Met?		NO										

Summary of Warrants

Spot Number:	0		
Major Street:	Jefferson St	Minor Street:	Independence St
Intersection:	Jefferson St at Independence St		
City/Twp:	Tipton, IN		
Date Performed:	2/28/2023	Performed By:	WBS
Date Volumes Collected:	2/22/2023		

Warrant	Condition	Is Warrant Met
Data Validation Error		NO
WARRANT 1: Eight-Hour Vehicular Volume		NO
	Condition A	NO
	Condition B	NO
	Condition A&B	N/A
WARRANT 2: Four-Hour Vehicular Volume	(70%)	NO
WARRANT 3: Peak-Hour Vehicular Volume	(70%)	NO
	Condition A	NO
	Condition B	NO
WARRANT 4: Pedestrian Volume	(70%)	NO
	Four Hour	N/A
	Peak Hour	N/A
	(Threshold)	HAWK
	(Threshold)	RRFB
WARRANT 5: School Crossing		NO
WARRANT 6: Coordinated Signal System		NO
WARRANT 7: Crash Experience		NO
	Condition A	NO
	Condition B	NO
WARRANT 8: Roadway Network		NO
WARRANT 9: Intersection Near a Grade Crossing		#N/A
Issue to Be Addressed by Signalization:		
0		

Indiana Manual of Uniform Traffic Control Devices

Worksheet for Signal Warrants (Section 4C)

WARRANT 1: Eight-Hour Vehicular Volume

Intersection:	Jefferson St @ Independence St		
Date	2/28/2023	by	WBS

2	: No. of Lanes on Major St?
1	: No. of Lanes on Minor St?
25	: Speed limit or 85th Percentile? (MPH)
YES	: Is the intersection within an Isolated community?
5275	: if answer 4 is Yes, then what is the of the population isolated community?
NO	: Have other remedial measures been tried?

USE 70% WARRANTS 1A AND 1B. DO NOT USE COMBINATION OF A & B

	Major Volume (Both Apr.)	Minor Volume (One Apr.)	Condition A Major Volume	Condition A Minor Volume	Warrant Condition A Met?	Condition B Major Volume	Condition B Minor Volume	Warrant Condition B Met?	Combination Major A	Combination Minor A	Combination Major B	Combination Minor B	Warrant Condition A&B met?
Time	E-W	N-S											
00:01 - 01:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
01:00 - 02:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
02:00 - 03:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
03:00 - 04:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
04:00 - 05:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
05:00 - 06:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
06:00 - 07:00	424	22	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
07:00 - 08:00	589	41	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
08:00 - 09:00	498	33	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
09:00 - 10:00	566	48	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
10:00 - 11:00	492	41	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
11:00 - 12:00	574	64	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
12:00 - 13:00	622	65	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
13:00 - 14:00	508	55	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
14:00 - 15:00	702	40	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
15:00 - 16:00	744	70	420	105	NO	630	53	YES	N/A	N/A	N/A	N/A	N/A
16:00 - 17:00	656	59	420	105	NO	630	53	YES	N/A	N/A	N/A	N/A	N/A
17:00 - 18:00	591	59	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
18:00 - 19:00	239	30	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
19:00 - 20:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
20:00 - 21:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
21:00 - 22:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
22:00 - 23:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A
23:00 - 00:00	0	0	420	105	NO	630	53	NO	N/A	N/A	N/A	N/A	N/A

Number of Hours that met the warrant 1A =
 Number of Hours that met the warrant 1B =
 Number of Hours that met the warrant 1 A & B =

A. Is the Minimum Vehicular Volume Warrant Met? (Condition A)	NO
B. Is the Interruption of Continuous Traffic Met? (Condition B)	NO
C. Combination of Warrants A and B Criteria Met?	N/A

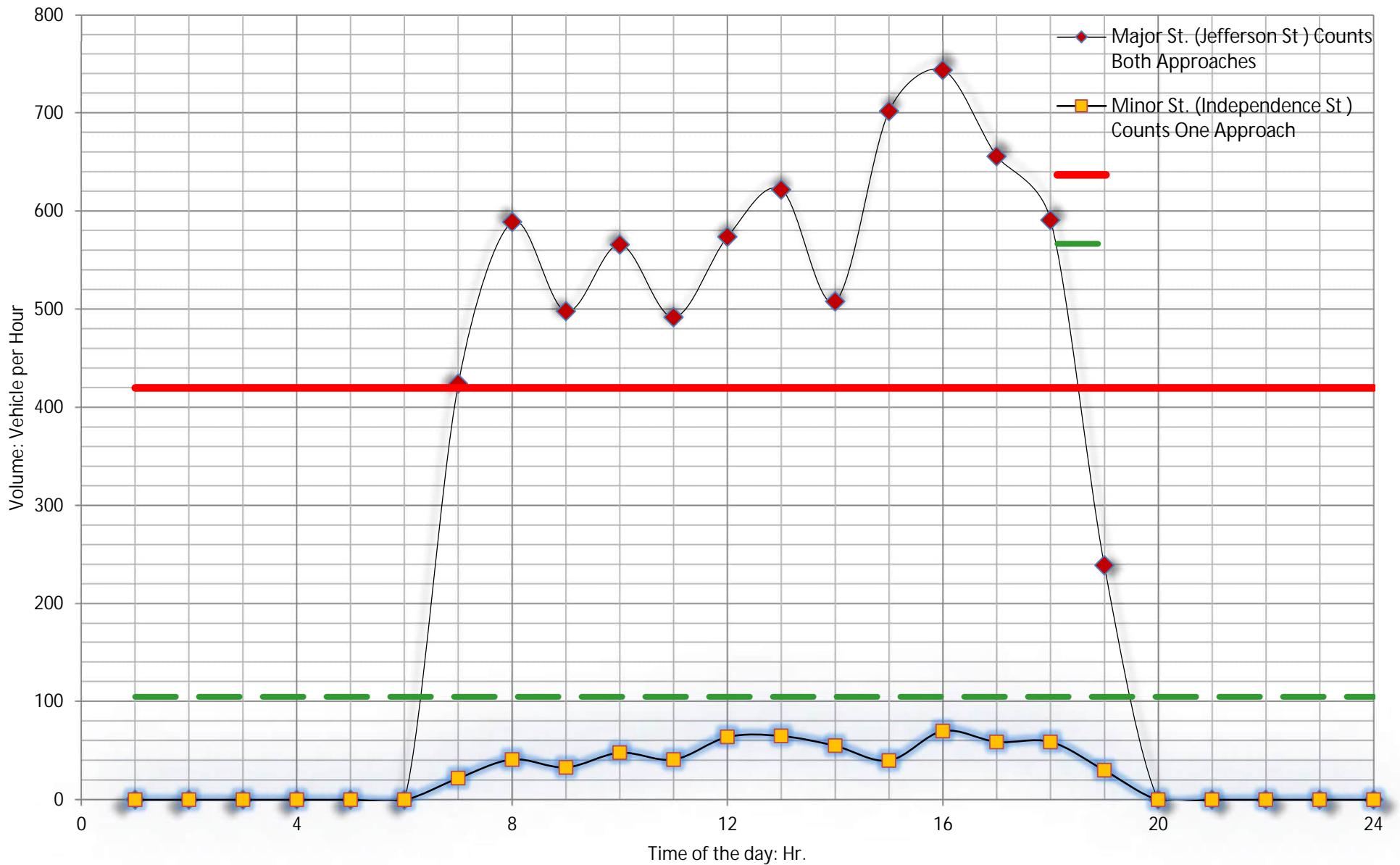


FIGURE 1: WARRANT 1A

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? **NO**

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? **YES**

Spot Number:

Jefferson St @ Independence St

NO. OF LANES ON MAJOR ST.? **2**

NO. OF LANES ON MINOR ST.? **1**

Number of Hours that met the Warrant: **0**

Does this intersection meet Warrant 1A for signal installation? **NO**

Data Collection Date: **2/22/2023**

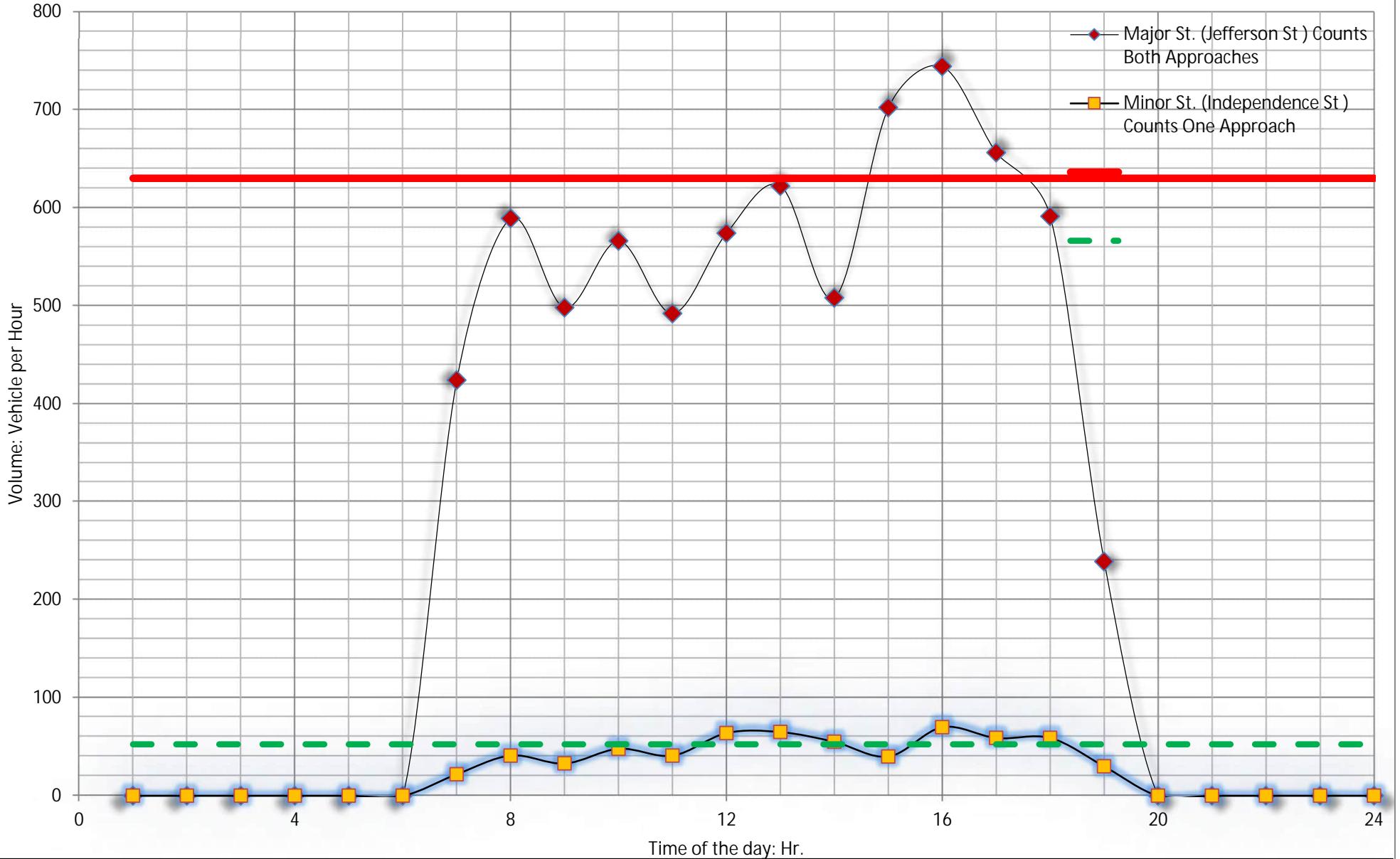


FIGURE 1: WARRANT 1B

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? NO

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? YES

Spot Number:

Jefferson St @ Independence St

NO. OF LANES ON MAJOR ST.? **2**

NO. OF LANES ON MINOR ST.? **1**

Number of Hours that met the Warrant: **2**

Does this intersection meet Warrant 1B **NO** for signal installation?

Data Collection Date: **2/22/2023**

Indiana Manual of Uniform Traffic Control Devices												
Worksheet for Signal Warrants (Section 4C)												
WARRANT 3 B(70%): Peak-Hour Vehicular Volume												
Spot Number:	0											
Intersection:	Jefferson St @ Independence St											
Date	2/28/2023	by WBS										
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td style="width: 20px; text-align: center;">2</td><td>: No. of Lanes on Major St.</td></tr> <tr><td style="text-align: center;">1</td><td>: No. of Lanes on Minor St.</td></tr> <tr><td style="text-align: center;">25</td><td>: Speed limit or 85th Percentile? (MPH)</td></tr> <tr><td style="text-align: center;">YES</td><td>: Is the intersection within an Isolated community?</td></tr> <tr><td style="text-align: center;">5275</td><td>: What is the of the population isolated community?</td></tr> </table>			2	: No. of Lanes on Major St.	1	: No. of Lanes on Minor St.	25	: Speed limit or 85th Percentile? (MPH)	YES	: Is the intersection within an Isolated community?	5275	: What is the of the population isolated community?
2	: No. of Lanes on Major St.											
1	: No. of Lanes on Minor St.											
25	: Speed limit or 85th Percentile? (MPH)											
YES	: Is the intersection within an Isolated community?											
5275	: What is the of the population isolated community?											
<p>MINOR STREET HIGHER VOLUME APPROACH-VPH</p> <p>MAJOR STREET - TOTAL OF BOTH APPROACHES - VEHICLES PER HOUR (VPH)</p>												
<input type="checkbox"/> How Many Hours Are Met 0												
<input type="checkbox"/> Is Warrant (70%) Met? NO												

Summary of Warrants

Spot Number:	0				
Major Street:	Main St	Minor Street:	Madison St		
Intersection:	Main St at Madison St				
City/Twp:	Tipton, IN				
Date Performed:	2/28/2023	Performed By:	WBS		
Date Volumes Collected:	2/22/2023				
Warrant	Condition	Is Warrant Met			
Data Validation Error		NO			
WARRANT 1: Eight-Hour Vehicular Volume		NO			
	Condition A	NO			
	Condition B	NO			
	Condition A&B	N/A			
WARRANT 2: Four-Hour Vehicular Volume	(70%)	NO			
WARRANT 3: Peak-Hour Vehicular Volume	(70%)	#N/A			
	Condition A	#N/A			
	Condition B	NO			
WARRANT 4: Pedestrian Volume	(70%)	NO			
	Four Hour	N/A			
	Peak Hour	N/A			
	(Threshold)	HAWK	NO		
	(Threshold)	RRFB	NO		
WARRANT 5: School Crossing		NO			
WARRANT 6: Coordinated Signal System		NO			
WARRANT 7: Crash Experience		NO			
	Condition A	NO			
	Condition B	NO			
WARRANT 8: Roadway Network		NO			
WARRANT 9: Intersection Near a Grade Crossing		#N/A			
Issue to Be Addressed by Signalization:					
0					

Indiana Manual of Uniform Traffic Control Devices

Worksheet for Signal Warrants (Section 4C)

WARRANT 1: Eight-Hour Vehicular Volume

Intersection:	Main St @ Madison St	
Date	2/28/2023	by WBS

1	: No. of Lanes on Major St?
1	: No. of Lanes on Minor St?
30	: Speed limit or 85th Percentile? (MPH)
YES	: Is the intersection within an Isolated community?
5275	: if answer 4 is Yes, then what is the of the population isolated community?
NO	: Have other remedial measures been tried?

USE 70% WARRANTS 1A AND 1B. DO NOT USE COMBINATION OF A & B

	Major Volume (Both Apr.)	Minor Volume (One Apr.)	Condition A Major Volume	Condition A Minor Volume	Warrant Condition A Met?	Condition B Major Volume	Condition B Minor Volume	Warrant Condition B Met?	Combination Major A	Combination Minor A	Combination Major B	Combination Minor B	Warrant Condition A&B met?
Time	N-S	E-W											
00:01 - 01:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
01:00 - 02:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
02:00 - 03:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
03:00 - 04:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
04:00 - 05:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
05:00 - 06:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
06:00 - 07:00	153	10	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
07:00 - 08:00	437	38	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
08:00 - 09:00	294	20	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
09:00 - 10:00	254	37	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
10:00 - 11:00	246	30	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
11:00 - 12:00	296	48	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
12:00 - 13:00	299	39	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
13:00 - 14:00	259	49	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
14:00 - 15:00	274	33	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
15:00 - 16:00	370	52	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
16:00 - 17:00	369	54	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
17:00 - 18:00	449	77	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
18:00 - 19:00	223	26	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
19:00 - 20:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
20:00 - 21:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
21:00 - 22:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
22:00 - 23:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A
23:00 - 00:00	0	0	350	105	NO	525	53	NO	N/A	N/A	N/A	N/A	N/A

Number of Hours that met the warrant 1A = 0

Number of Hours that met the warrant 1B = 0

Number of Hours that met the warrant 1 A & B = 0

A. Is the Minimum Vehicular Volume Warrant Met? (Condition A)	NO
B. Is the Interruption of Continuous Traffic Met? (Condition B)	NO
C. Combination of Warrants A and B Criteria Met?	N/A

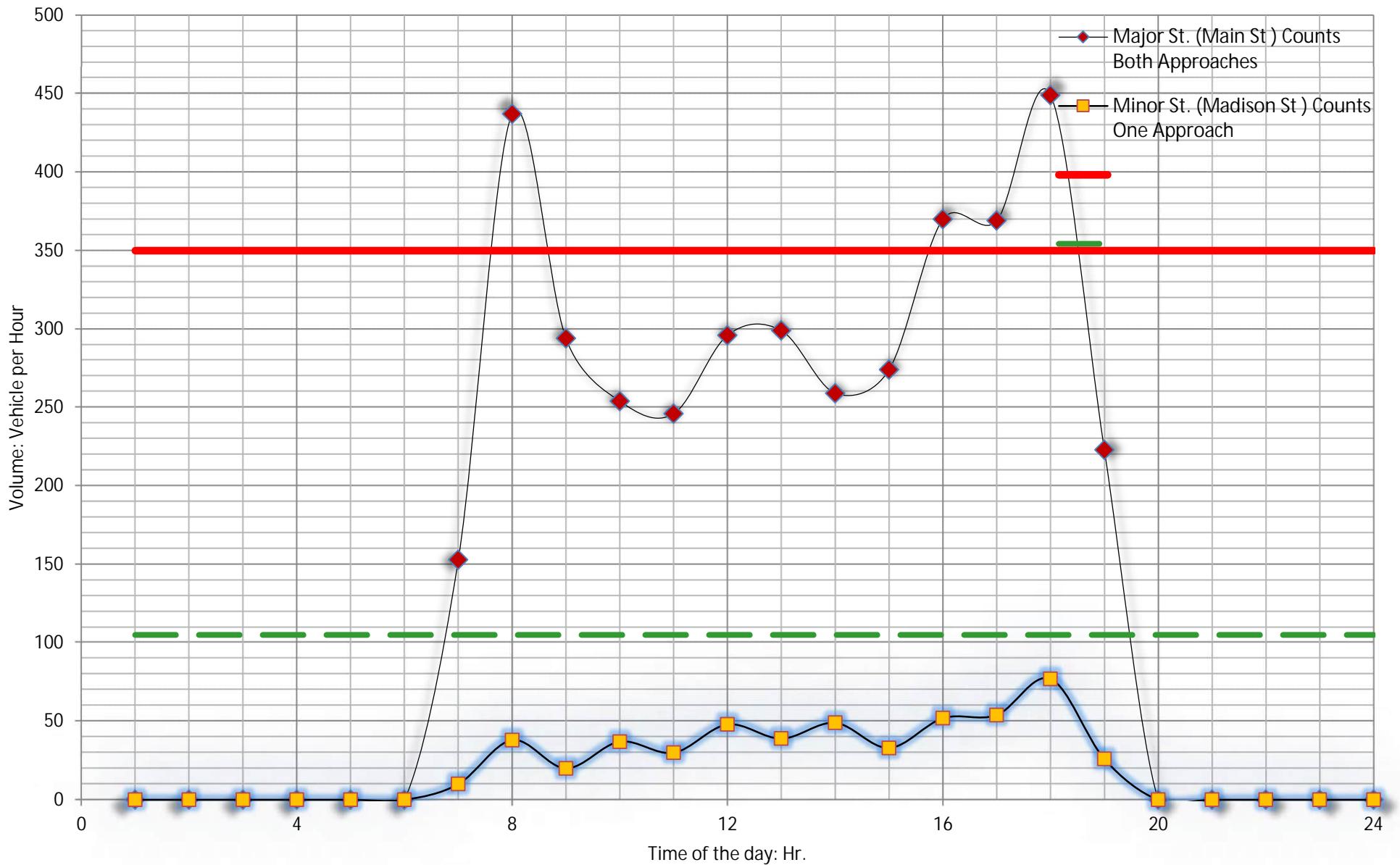


FIGURE 1: WARRANT 1A

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? **NO**

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? **YES**

Spot Number:

Main St @ Madison St

NO. OF LANES ON MAJOR ST.? **1**

NO. OF LANES ON MINOR ST.? **1**

Number of Hours that met the Warrant: **0**

Does this intersection meet Warrant 1A for signal installation? **NO**

Data Collection Date: **2/22/2023**

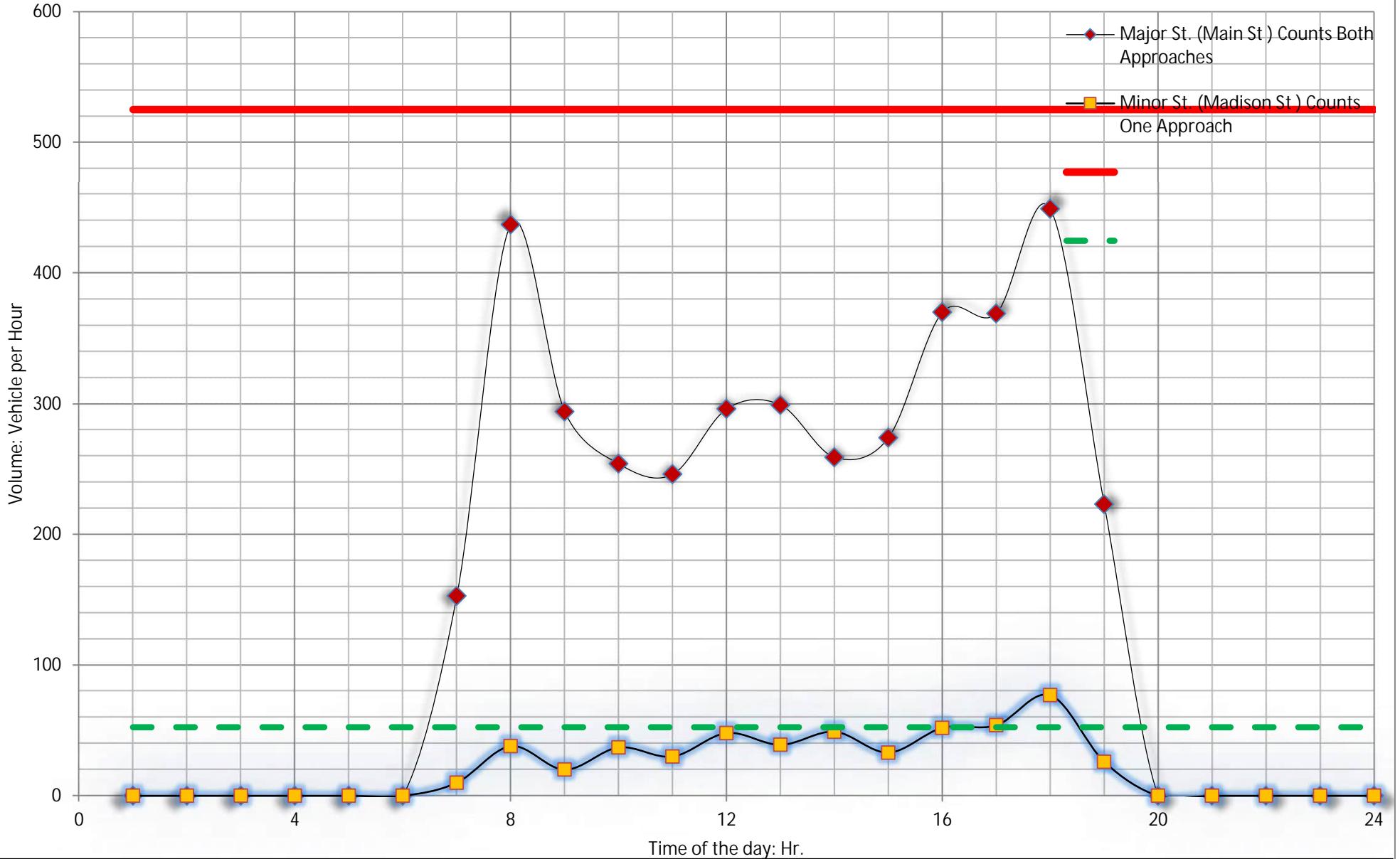


FIGURE 1: WARRANT 1B

IS THERE A REDUCTION IN THE WARRANT THRESHOLDS TO 70% ...

1- DUE TO SPEED? NO

2- DUE TO ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000? YES

Spot Number:

Main St @ Madison St

NO. OF LANES ON MAJOR ST.? 1
NO. OF LANES ON MINOR ST.? 1

Number of Hours that met the Warrant: 0

Does this intersection meet Warrant 1B NO for signal installation?

Data Collection Date: 2/22/2023

Indiana Manual of Uniform Traffic Control Devices												
Worksheet for Signal Warrants (Section 4C)												
WARRANT 3 B(70%): Peak-Hour Vehicular Volume												
Spot Number:	0											
Intersection:	Main St @ Madison St											
Date	2/28/2023	by WBS										
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr><td style="width: 10%;">1</td><td>: No. of Lanes on Major St.</td></tr> <tr><td>1</td><td>: No. of Lanes on Minor St.</td></tr> <tr><td>30</td><td>: Speed limit or 85th Percentile? (MPH)</td></tr> <tr><td>YES</td><td>: Is the intersection within an Isolated community?</td></tr> <tr><td>5275</td><td>: What is the of the population isolated community?</td></tr> </table>			1	: No. of Lanes on Major St.	1	: No. of Lanes on Minor St.	30	: Speed limit or 85th Percentile? (MPH)	YES	: Is the intersection within an Isolated community?	5275	: What is the of the population isolated community?
1	: No. of Lanes on Major St.											
1	: No. of Lanes on Minor St.											
30	: Speed limit or 85th Percentile? (MPH)											
YES	: Is the intersection within an Isolated community?											
5275	: What is the of the population isolated community?											
<p style="text-align: center; margin-top: 10px;"> MINOR STREET HIGHER VOLUME APPROACH-VPH MAJOR STREET - TOTAL OF BOTH APPROACHES - VEHICLES PER HOUR (VPH) </p>												
How Many Hours Are Met Is Warrant (70%) Met?		0 NO										

APPENDIX D

SYNCHRO RESULTS TWO-WAY STOP-CONTROLLED

Intersection

Int Delay, s/veh 2.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗	
Traffic Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Future Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	50	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	75	75	84	84	84	68	68	68	50	50	50
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	325	33	13	290	10	34	10	24	10	32	6

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	300	0	0	358	0	0	690	676	342	688	687	295
Stage 1	-	-	-	-	-	-	350	350	-	321	321	-
Stage 2	-	-	-	-	-	-	340	326	-	367	366	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1261	-	-	1201	-	-	359	375	701	360	370	744
Stage 1	-	-	-	-	-	-	666	633	-	691	652	-
Stage 2	-	-	-	-	-	-	675	648	-	653	623	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1261	-	-	1201	-	-	329	370	701	337	365	744
Mov Cap-2 Maneuver	-	-	-	-	-	-	329	370	-	337	365	-
Stage 1	-	-	-	-	-	-	664	631	-	689	645	-
Stage 2	-	-	-	-	-	-	629	641	-	619	621	-

Approach	EB	WB	NB	SB				
HCM Control Delay, s	0.1	0.3	15.4	15.7				
HCM LOS			C	C				
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Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	412	1261	-	-	1201	-	-	383
HCM Lane V/C Ratio	0.164	0.003	-	-	0.011	-	-	0.125
HCM Control Delay (s)	15.4	7.9	-	-	8	-	-	15.7
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.6	0	-	-	0	-	-	0.4

Intersection

Int Delay, s/veh 14.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑		↑	↑	
Traffic Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Future Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	150	-	-	190	-	-	30	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	86	86	86	59	59	59	70	70	70
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	18	240	86	113	249	21	54	98	112	23	167	30

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	270	0	0	326	0	0	903	815	283	910	848	260
Stage 1	-	-	-	-	-	-	319	319	-	486	486	-
Stage 2	-	-	-	-	-	-	584	496	-	424	362	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1293	-	-	1234	-	-	258	312	756	255	298	779
Stage 1	-	-	-	-	-	-	693	653	-	563	551	-
Stage 2	-	-	-	-	-	-	498	545	-	608	625	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1293	-	-	1234	-	-	115	279	756	146	267	779
Mov Cap-2 Maneuver	-	-	-	-	-	-	115	279	-	146	267	-
Stage 1	-	-	-	-	-	-	683	644	-	555	500	-
Stage 2	-	-	-	-	-	-	290	495	-	433	616	-

Approach	EB	WB	NB	SB							
HCM Control Delay, s	0.4	2.4	30	37.7							
HCM LOS		D	E								
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Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	
Capacity (veh/h)	115	420	1293	-	-	1234	-	-	146	297	
HCM Lane V/C Ratio	0.472	0.5	0.014	-	-	0.091	-	-	0.157	0.664	
HCM Control Delay (s)	61.6	21.9	7.8	-	-	8.2	-	-	34.2	38.1	
HCM Lane LOS	F	C	A	-	-	A	-	-	D	E	
HCM 95th %tile Q(veh)	2.1	2.7	0	-	-	0.3	-	-	0.5	4.4	

Intersection

Int Delay, s/veh 2.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Future Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	58	58	75	75	75	57	57	57	66	66	66
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	12	57	17	5	9	35	279	25	8	400	20

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	795	800	410	823	798	292	420	0	0	304	0	0
Stage 1	426	426	-	362	362	-	-	-	-	-	-	-
Stage 2	369	374	-	461	436	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	305	318	642	292	319	747	1139	-	-	1257	-	-
Stage 1	606	586	-	657	625	-	-	-	-	-	-	-
Stage 2	651	618	-	581	580	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	287	304	642	249	305	747	1139	-	-	1257	-	-
Mov Cap-2 Maneuver	287	304	-	249	305	-	-	-	-	-	-	-
Stage 1	584	581	-	633	602	-	-	-	-	-	-	-
Stage 2	614	595	-	514	575	-	-	-	-	-	-	-

Approach	EB	WB			NB		SB	
HCM Control Delay, s	13.1	17.5			0.9		0.1	
HCM LOS	B	C						
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Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1139	-	-	516	321	1257	-	-
HCM Lane V/C Ratio	0.031	-	-	0.14	0.1	0.006	-	-
HCM Control Delay (s)	8.3	0	-	13.1	17.5	7.9	0	-
HCM Lane LOS	A	A	-	B	C	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.5	0.3	0	-	-

Intersection

Int Delay, s/veh

2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Future Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	100	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	94	94	94	61	61	61	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	6	295	7	18	346	15	10	13	21	12	19	21

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	361	0	0	302	0	0	721	708	299	718	704	354
Stage 1	-	-	-	-	-	-	311	311	-	390	390	-
Stage 2	-	-	-	-	-	-	410	397	-	328	314	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1198	-	-	1259	-	-	343	360	741	344	361	690
Stage 1	-	-	-	-	-	-	699	658	-	634	608	-
Stage 2	-	-	-	-	-	-	619	603	-	685	656	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1198	-	-	1259	-	-	315	353	741	320	354	690
Mov Cap-2 Maneuver	-	-	-	-	-	-	315	353	-	320	354	-
Stage 1	-	-	-	-	-	-	696	655	-	631	599	-
Stage 2	-	-	-	-	-	-	573	595	-	649	653	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.2	0.4			13.7			14.5			
HCM LOS					B			B			
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Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBLn1		
Capacity (veh/h)	456	1198	-	-	1259	-	-	-	429		
HCM Lane V/C Ratio	0.097	0.005	-	-	0.014	-	-	-	0.121		
HCM Control Delay (s)	13.7	8	-	-	7.9	-	-	-	14.5		
HCM Lane LOS	B	A	-	-	A	-	-	-	B		
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	-	0.4		

Intersection

Int Delay, s/veh 2.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Future Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	50	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	89	89	89	80	80	80	69	69	69
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	338	18	43	311	22	36	11	41	12	20	4

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	333	0	0	356	0	0	781	780	347	795	778	322
Stage 1	-	-	-	-	-	-	361	361	-	408	408	-
Stage 2	-	-	-	-	-	-	420	419	-	387	370	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1226	-	-	1203	-	-	312	327	696	305	328	719
Stage 1	-	-	-	-	-	-	657	626	-	620	597	-
Stage 2	-	-	-	-	-	-	611	590	-	637	620	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1226	-	-	1203	-	-	285	313	696	270	314	719
Mov Cap-2 Maneuver	-	-	-	-	-	-	285	313	-	270	314	-
Stage 1	-	-	-	-	-	-	653	622	-	616	576	-
Stage 2	-	-	-	-	-	-	565	569	-	585	616	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.2	0.9			16.6			17.7			
HCM LOS					C			C			
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	399	1226	-	-	1203	-	-	319			
HCM Lane V/C Ratio	0.222	0.006	-	-	0.035	-	-	0.114			
HCM Control Delay (s)	16.6	8	-	-	8.1	-	-	17.7			
HCM Lane LOS	C	A	-	-	A	-	-	C			
HCM 95th %tile Q(veh)	0.8	0	-	-	0.1	-	-	0.4			

Intersection

Int Delay, s/veh 10.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑		↑	↑	
Traffic Vol, veh/h	26	323	38	66	259	32	58	70	96	22	71	19
Future Vol, veh/h	26	323	38	66	259	32	58	70	96	22	71	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	150	-	-	190	-	-	30	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	89	89	89	81	81	84	69	69	69
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	27	336	40	74	291	36	72	86	114	32	103	28

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	327	0	0	376	0	0	933	885	356	967	887	309
Stage 1	-	-	-	-	-	-	410	410	-	457	457	-
Stage 2	-	-	-	-	-	-	523	475	-	510	430	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1233	-	-	1182	-	-	246	284	688	234	283	731
Stage 1	-	-	-	-	-	-	619	595	-	583	568	-
Stage 2	-	-	-	-	-	-	537	557	-	546	583	-
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	1233	-	-	1182	-	-	154	260	688	136	259	731
Mov Cap-2 Maneuver	-	-	-	-	-	-	154	260	-	136	259	-
Stage 1	-	-	-	-	-	-	605	582	-	570	532	-
Stage 2	-	-	-	-	-	-	391	522	-	379	570	-

Approach	EB	WB	NB	SB							
HCM Control Delay, s	0.5	1.5	29	28.6							
HCM LOS			D	D							
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Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	
Capacity (veh/h)	154	403	1233	-	-	1182	-	-	136	300	
HCM Lane V/C Ratio	0.465	0.498	0.022	-	-	0.063	-	-	0.234	0.435	
HCM Control Delay (s)	47.2	22.5	8	-	-	8.2	-	-	39.4	25.9	
HCM Lane LOS	E	C	A	-	-	A	-	-	E	D	
HCM 95th %tile Q(veh)	2.2	2.7	0.1	-	-	0.2	-	-	0.9	2.1	

Intersection

Int Delay, s/veh 3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Future Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	63	63	63	82	82	82	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	25	23	36	19	6	14	24	246	21	15	149	21

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	505	505	160	524	505	257	170	0	0	267	0	0
Stage 1	190	190	-	305	305	-	-	-	-	-	-	-
Stage 2	315	315	-	219	200	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	478	470	885	464	470	782	1407	-	-	1297	-	-
Stage 1	812	743	-	705	662	-	-	-	-	-	-	-
Stage 2	696	656	-	783	736	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	453	454	885	417	454	782	1407	-	-	1297	-	-
Mov Cap-2 Maneuver	453	454	-	417	454	-	-	-	-	-	-	-
Stage 1	796	733	-	691	649	-	-	-	-	-	-	-
Stage 2	663	643	-	718	726	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	12.4	12.7	0.6	0.6
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1407	-	-	572	509	1297	-	-
HCM Lane V/C Ratio	0.017	-	-	0.146	0.078	0.012	-	-
HCM Control Delay (s)	7.6	0	-	12.4	12.7	7.8	0	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.5	0.3	0	-	-

Intersection														
Int Delay, s/veh	2.3													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗			
Traffic Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24		
Future Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop		
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None		
Storage Length	100	-	-	75	-	-	-	-	-	-	-	-		
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-		
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-		
Peak Hour Factor	87	87	87	93	93	93	92	92	92	75	75	75		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2		
Mvmt Flow	20	461	16	16	361	26	14	11	35	13	19	32		
Major/Minor														
Major1		Major2		Minor1		Minor2								
Conflicting Flow All	387	0	0	477	0	0	941	928	469	938	923	374		
Stage 1	-	-	-	-	-	-	509	509	-	406	406	-		
Stage 2	-	-	-	-	-	-	432	419	-	532	517	-		
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22		
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-		
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-		
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318		
Pot Cap-1 Maneuver	1171	-	-	1085	-	-	243	268	594	244	270	672		
Stage 1	-	-	-	-	-	-	547	538	-	622	598	-		
Stage 2	-	-	-	-	-	-	602	590	-	531	534	-		
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-		
Mov Cap-1 Maneuver	1171	-	-	1085	-	-	214	259	594	217	261	672		
Mov Cap-2 Maneuver	-	-	-	-	-	-	214	259	-	217	261	-		
Stage 1	-	-	-	-	-	-	538	529	-	611	589	-		
Stage 2	-	-	-	-	-	-	547	581	-	481	525	-		
Approach														
EB			WB			NB			SB					
HCM Control Delay, s	0.3		0.3		17		17.4							
HCM LOS				C			C							
Minor Lane/Major Mvmt														
NBLn1		EBL	EBT	EBR	WBL	WBT	WBR	SBLn1						
Capacity (veh/h)	359	1171	-	-	1085	-	-	354						
HCM Lane V/C Ratio	0.167	0.017	-	-	0.015	-	-	0.181						
HCM Control Delay (s)	17	8.1	-	-	8.4	-	-	17.4						
HCM Lane LOS	C	A	-	-	A	-	-	C						
HCM 95th %tile Q(veh)	0.6	0.1	-	-	0	-	-	0.7						

APPENDIX E

INDIANA DESIGN MANUAL SIGHT DISTANCE REQUIREMENTS

46-10.03 Stop Control

Where traffic on the minor road of an intersection is controlled by a “Stop” sign, the driver of the vehicle on the minor road must have sufficient sight distance for a safe departure from the stopped position assuming that the approaching vehicle comes into view as the stopped vehicle begins its departure. The location of the eye should be as described in Section [46-10.03\(01\)](#).

46-10.03(01) Departure Sight Triangle and Time Gap

The departure sight triangle for an intersection with stop control on the minor road must consider the situations as follows:

1. left turn from the minor road;
2. right turn from the minor road; and
3. crossing the major road from minor-road approach.

A departure sight triangle for traffic approaching from either the right or left, like that shown in Figure [46-10F](#), Departure Sight Triangles, should be provided for a left turn from the minor road onto the major road for each stop-controlled approach.

Field observations of the gaps in major-road traffic actually accepted by drivers turning onto the major road have shown that the values shown in Figure [46-10G](#), Intersection Sight Distance for Stop-Controlled Intersection, provide sufficient time for the minor-road vehicle to accelerate from a stop and complete a left turn without unduly interfering with major-road traffic operations.

The intersection sight distance in both directions should be equal to the distance traveled at the design speed of the major road during a period of time equal to the time gap. At a minimum, ISD should be checked for both a passenger car and a single unit truck turning from the minor-road approach. Where a substantial volume of heavy vehicles enter the major road, the use of combination trucks should be considered.

No adjustment is needed for the major-road grade. However, if the minor-road design vehicle is a truck and the intersection is located near a sag vertical curve with a grade over 3%, an adjustment of the intersection sight distance should be considered.

Figure [46-10G](#) provides the criteria for intersection sight distance in both directions for a vehicle turning left.

Intersection sight distance for a left turn at a divided-highway intersection should consider multiple design vehicles and median width. If the design vehicle used to determine sight distance for a divided-highway intersection is larger than a passenger car, sight distance for a left turn will need to be checked for that selected design vehicle and for smaller design vehicles as well. If the divided-highway median is wide enough to store the design vehicle with a clearance to the through lanes of 3 ft at both ends of the vehicle, no separate analysis for the departure sight triangle for a left turn is needed on the minor-road approach for the near roadway to the left.

If the design vehicle can be stored in the median with adequate clearance to the through lanes, a departure sight triangle to the right for a left turn should be provided for that design vehicle turning left from the median roadway. Where the median is not wide enough to store the design vehicle, a departure sight triangle should be provided for that design vehicle to turn left from the minor-road approach. The median width should be considered in determining the number of lanes to be crossed. The median width should be converted to an equivalent number of lanes.

The sight triangle for a left or right turn onto the major road will also provide more than adequate sight distance for a minor-road vehicle to cross the major road. However, the intersection sight distance for a crossing maneuver must be checked for the situations as follows:

1. where left or right turns are not permitted from a particular approach and the crossing maneuver is the only legal maneuver;
2. where the crossing vehicle would cross the equivalent width of more than 6 lanes; or
3. where a substantial volume of heavy vehicles cross the highway, and steep grades that might slow such vehicles while their back portions are still in the intersection are present on the departure roadway on the far side of the intersection.

The time gap shown in Figure [46-10H\(1\)](#), Time Gap for Crossing Maneuver, may be used for the crossing-maneuver check.

Figure [46-10H](#), Intersection Sight Distance for Passenger Car to Turn Right provides the intersection sight distance for a passenger car making a right turn from a stop or a crossing maneuver.

At a divided-highway intersection, depending on the median width and the length of the design vehicle, intersection sight distance may need to be considered for crossing both roadways of a divided highway or for crossing the near lanes only and stopping in the median before proceeding.

The ISD value will establish one leg of the sight triangle which needs to be visible to the entering vehicle. The leg on the stop-controlled road or street will be determined by the assumed location of the eye. This is established as 18 ft behind the edge of the travel lane for a new or reconstruction project, or 14.5 ft for a 3R project (see Figure [46-10F](#), Departure Sight Triangle).

46-10.03(02) Measures to Improve Intersection Sight Distance

The available ISD should be checked using the parameters described above. If the line of sight falls above a bridge railing and guardrail and the ISD value from Figure [46-10G](#) is provided, no further investigation is needed. If the line of sight is restricted by the bridge railing, guardrail, or other obstruction, or the horizontal and vertical alignment of the major road and the ISD value is not available, one or more of the modifications, or a combination of them, should be evaluated to achieve the intersection sight distance as follows:

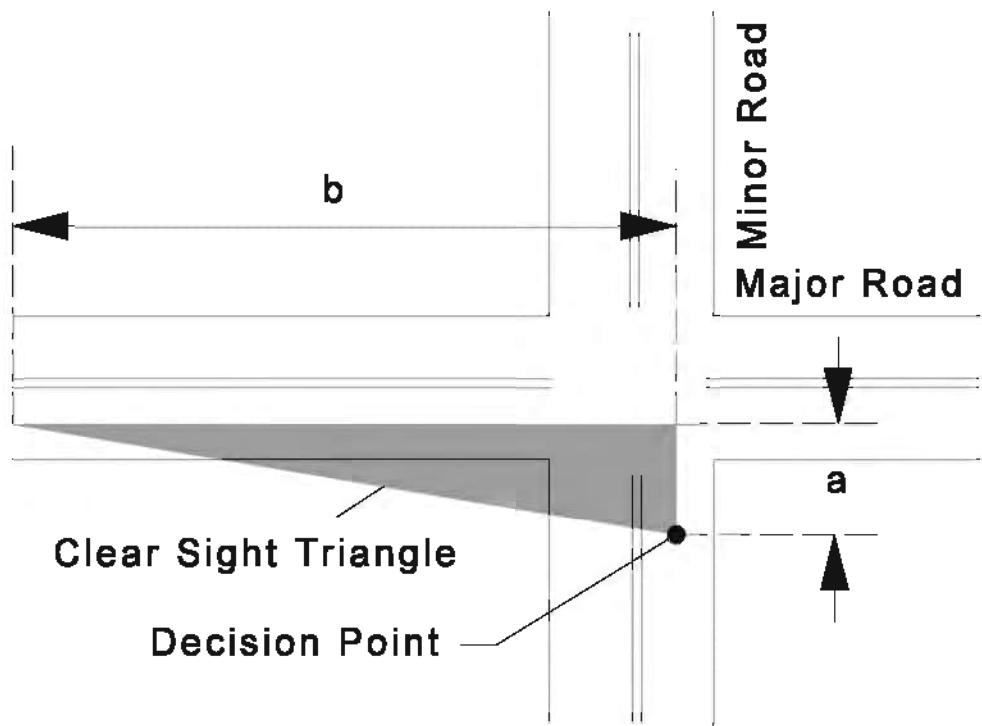
1. relocate the minor road or drive farther from the end of the bridge;
2. widen the structure on the side where the railing is restricting the line of sight;
3. flare the approach guardrail;
4. revise the grades on the major road or the minor road or drive;
5. remove the obstruction that is restricting sight distance;
6. close the minor road or drive;
7. make the minor road or drive one-way away from the major road; or
8. review other measures that may be practical at a particular location.

If intersection sight distance along the major road cannot be achieved, consideration should be given to installing advance intersection signing with advisory speed plates.

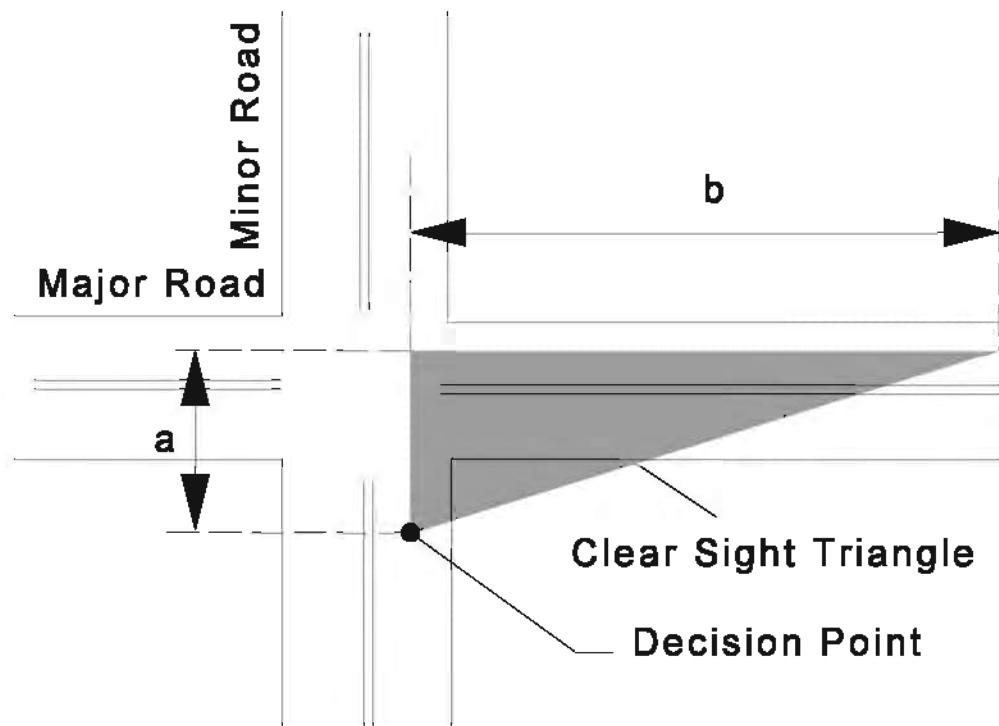
46-10.04 Left Turn From the Major Road

Each location along the major road from which a vehicle is permitted to turn left across opposing traffic, including an intersection or drive, should have sufficient sight distance to accommodate the left-turn maneuver. A left-turning driver needs sufficient sight distance to decide when it is safe to turn left across the lanes used by opposing traffic. Sight distance design should be based on a left turn by a stopped vehicle, since a vehicle that turns left without stopping would need less sight distance. The sight distance along the major road to accommodate a left turn is the distance traversed at the design speed of the major road in the travel time for the design vehicle shown in Figure [46-10I](#).

The figure also includes appropriate adjustment factors for the number of major-road lanes to be crossed by the turning vehicle. The unadjusted time gap shown in Figure [46-10I](#) for a passenger car was used to develop the sight distance shown in Figure [46-10J](#).



Clear Sight Triangle for Viewing Traffic Approaching from the Left.



Clear Sight Triangle for Viewing Traffic Approaching from the Right.

DEPARTURE SIGHT TRIANGLES

Figure 46-10F

V_{major} (mph)	Passenger Car				Single-Unit Truck		Combination Truck	
	Local Road		Collector or Arterial		t_g (s)	ISD (ft)	t_g (s)	ISD (ft)
	t_g (s)	ISD (ft)	t_g (s)	ISD (ft)				
15	7.5	170	7.5	170	9.5	210	11.5	260
20	7.5	220	7.5	220	9.5	280	11.5	340
25	7.5	280	7.5	280	9.5	350	11.5	430
30	7.5	330	7.5	330	9.5	420	11.5	510
35	7.5	390	7.5	390	9.5	490	11.5	600
40	7.5	440	7.5	440	9.5	560	11.5	680
45	7.5	500	7.5	500	9.5	630	11.5	760
50	7.5	550	8.5	630	10.5	780	12.5	920
55	7.5	610	9.0	730	11.0	890	13.0	1060
60	7.5	670	9.5	840	11.5	1020	13.5	1190
65	7.5	720	10.0	960	12.0	1150	14.0	1340
70	7.5	780	10.0	1030	12.0	1240	14.0	1440

V_{major} = Design speed of major road

t_g = Time gap for minor road vehicle to enter major road

ISD = Intersection sight distance (length of leg of sight triangle along major road)

ISD is shown for a stopped vehicle to turn left onto a two-lane highway with approach grades of 3% or flatter. For other conditions, the time gap should be adjusted and the required ISD recalculated using the formula $ISD = 1.47 V_{major} t_g$.

For a left turn onto a two-way highway with more than two lanes, add 0.5 s for a passenger car, or 0.7 s for a truck for each additional lane from the left in excess of one, to be crossed by a turning vehicle.

For the minor-road approach, if its grade is an upgrade that is steeper than 3%, add 0.2 s for each percent grade for a left turn. The adjustment for the minor-road approach grade is required only if the rear wheels of the design vehicle would be on an upgrade steeper than 3%.

INTERSECTION SIGHT DISTANCE FOR STOP-CONTROLLED INTERSECTION

Figure 46-10G

Intersection Sight Distance For Passenger Car		
Design Speed (mph)	Calculated (ft)	Design (ft)
15	143.3	145
20	191.1	195
25	238.9	240
30	286.7	290
35	334.4	335
40	382.2	385
45	430.0	430
50	477.8	480
55	525.5	530
60	573.3	575
65	621.1	625
70	668.9	670

Note: Intersection sight distance shown is for a stopped passenger car to turn right onto or cross a two-lane highway with no median and grades of 3% or flatter. For other conditions, the time gap should be adjusted and the required sight distance recalculated.

INTERSECTION SIGHT DISTANCE FOR PASSENGER CAR TO TURN RIGHT FROM A STOP OR TO MAKE A CROSSING MANEUVER

Figure 46-10H

APPENDIX F

SYNCHRO RESULTS ALL-WAY STOP-CONTROLLED

Intersection

Intersection Delay, s/veh 11.9

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Future Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Peak Hour Factor	0.75	0.75	0.75	0.84	0.84	0.84	0.68	0.68	0.68	0.50	0.50	0.50
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	325	33	13	290	10	34	10	24	10	32	6
Number of Lanes	1	1	0	1	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			2		
HCM Control Delay	12.9			11.6			9.3			9.2		
HCM LOS	B			B			A			A		

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	50%	100%	0%	100%	0%	21%
Vol Thru, %	15%	0%	91%	0%	97%	67%
Vol Right, %	35%	0%	9%	0%	3%	12%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	46	3	269	11	252	24
LT Vol	23	3	0	11	0	5
Through Vol	7	0	244	0	244	16
RT Vol	16	0	25	0	8	3
Lane Flow Rate	68	4	359	13	300	48
Geometry Grp	2	7	7	7	7	2
Degree of Util (X)	0.104	0.006	0.504	0.021	0.428	0.075
Departure Headway (Hd)	5.528	5.631	5.062	5.667	5.141	5.642
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	643	633	710	629	696	629
Service Time	3.609	3.387	2.818	3.425	2.899	3.728
HCM Lane V/C Ratio	0.106	0.006	0.506	0.021	0.431	0.076
HCM Control Delay	9.3	8.4	12.9	8.5	11.7	9.2
HCM Lane LOS	A	A	B	A	B	A
HCM 95th-tile Q	0.3	0	2.9	0.1	2.2	0.2

Intersection

Intersection Delay, s/veh 16.1

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Future Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Peak Hour Factor	0.77	0.77	0.77	0.86	0.86	0.86	0.59	0.59	0.59	0.70	0.70	0.70
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	18	240	86	113	249	21	54	98	112	23	167	30
Number of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
Approach	EB		WB			NB			SB			
Opposing Approach	WB		EB			SB			NB			
Opposing Lanes	2		2			2			2			
Conflicting Approach Left	SB		NB			EB			WB			
Conflicting Lanes Left	2		2			2			2			
Conflicting Approach Right	NB		SB			WB			EB			
Conflicting Lanes Right	2		2			2			2			
HCM Control Delay	19.4		15.5			13.9			14.6			
HCM LOS	C		C			B			B			

Lane	NBLn1	NBLn2	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	100%	0%	100%	0%	100%	0%	100%	0%
Vol Thru, %	0%	47%	0%	74%	0%	92%	0%	85%
Vol Right, %	0%	53%	0%	26%	0%	8%	0%	15%
Sign Control	Stop							
Traffic Vol by Lane	32	124	14	251	97	232	16	138
LT Vol	32	0	14	0	97	0	16	0
Through Vol	0	58	0	185	0	214	0	117
RT Vol	0	66	0	66	0	18	0	21
Lane Flow Rate	54	210	18	326	113	270	23	197
Geometry Grp	7	7	7	7	7	7	7	7
Degree of Util (X)	0.119	0.408	0.038	0.615	0.233	0.515	0.051	0.402
Departure Headway (Hd)	7.888	6.993	7.491	6.792	7.443	6.876	7.966	7.343
Convergence, Y/N	Yes							
Cap	454	515	478	531	482	523	449	489
Service Time	5.643	4.747	5.241	4.541	5.194	4.627	5.722	5.099
HCM Lane V/C Ratio	0.119	0.408	0.038	0.614	0.234	0.516	0.051	0.403
HCM Control Delay	11.7	14.5	10.5	19.9	12.5	16.8	11.2	15
HCM Lane LOS	B	B	B	C	B	C	B	B
HCM 95th-tile Q	0.4	2	0.1	4.1	0.9	2.9	0.2	1.9

Intersection

Intersection Delay, s/veh 11.8

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖↗			↖↗			↖↗			↖↗	
Traffic Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Future Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Peak Hour Factor	0.58	0.58	0.58	0.75	0.75	0.75	0.57	0.57	0.57	0.66	0.66	0.66
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	12	57	17	5	9	35	279	25	8	400	20
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	8.9			9.1			11.3			12.9		
HCM LOS	A			A			B			B		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	10%	5%	54%	2%
Vol Thru, %	82%	17%	17%	94%
Vol Right, %	7%	79%	29%	5%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	193	42	24	282
LT Vol	20	2	13	5
Through Vol	159	7	4	264
RT Vol	14	33	7	13
Lane Flow Rate	339	72	32	427
Geometry Grp	1	1	1	1
Degree of Util (X)	0.437	0.105	0.051	0.541
Departure Headway (Hd)	4.651	5.212	5.686	4.559
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	772	681	624	786
Service Time	2.703	3.294	3.776	2.608
HCM Lane V/C Ratio	0.439	0.106	0.051	0.543
HCM Control Delay	11.3	8.9	9.1	12.9
HCM Lane LOS	B	A	A	B
HCM 95th-tile Q	2.2	0.4	0.2	3.3

Intersection

Intersection Delay, s/veh 11.7

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Future Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Peak Hour Factor	0.81	0.81	0.81	0.94	0.94	0.94	0.61	0.61	0.61	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	6	295	7	18	346	15	10	13	21	12	19	21
Number of Lanes	1	1	0	1	1	0	0	1	0	0	1	0
Approach	EB		WB			NB			SB			
Opposing Approach	WB		EB			NB			SB			NB
Opposing Lanes	2		2			1			1			1
Conflicting Approach Left	SB		NB			EB			WB			WB
Conflicting Lanes Left	1		1			2			2			2
Conflicting Approach Right	NB		SB			WB			EB			EB
Conflicting Lanes Right	1		1			2			2			2
HCM Control Delay	11.5		12.6			8.9			9			
HCM LOS	B		B			A			A			

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	22%	100%	0%	100%	0%	23%
Vol Thru, %	30%	0%	98%	0%	96%	36%
Vol Right, %	48%	0%	2%	0%	4%	41%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	27	5	245	17	339	39
LT Vol	6	5	0	17	0	9
Through Vol	8	0	239	0	325	14
RT Vol	13	0	6	0	14	16
Lane Flow Rate	44	6	302	18	361	52
Geometry Grp	2	7	7	7	7	2
Degree of Util (X)	0.066	0.01	0.428	0.028	0.503	0.079
Departure Headway (Hd)	5.407	5.616	5.096	5.554	5.022	5.438
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	657	636	705	643	714	654
Service Time	3.484	3.364	2.843	3.301	2.768	3.51
HCM Lane V/C Ratio	0.067	0.009	0.428	0.028	0.506	0.08
HCM Control Delay	8.9	8.4	11.6	8.5	12.8	9
HCM Lane LOS	A	A	B	A	B	A
HCM 95th-tile Q	0.2	0	2.2	0.1	2.9	0.3

Intersection

Intersection Delay, s/veh 12.3

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Future Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Peak Hour Factor	0.96	0.96	0.96	0.89	0.89	0.89	0.80	0.80	0.80	0.69	0.69	0.69
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	338	18	43	311	22	36	11	41	12	20	4
Number of Lanes	1	1	0	1	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			2		
HCM Control Delay	13.3			12.2			9.5			9.3		
HCM LOS	B			B			A			A		

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	41%	100%	0%	100%	0%	32%
Vol Thru, %	13%	0%	95%	0%	93%	56%
Vol Right, %	46%	0%	5%	0%	7%	12%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	71	7	341	38	297	25
LT Vol	29	7	0	38	0	8
Through Vol	9	0	324	0	277	14
RT Vol	33	0	17	0	20	3
Lane Flow Rate	89	7	355	43	334	36
Geometry Grp	2	7	7	7	7	2
Degree of Util (X)	0.137	0.012	0.512	0.068	0.478	0.06
Departure Headway (Hd)	5.541	5.73	5.191	5.707	5.155	5.949
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	640	621	689	624	694	606
Service Time	3.637	3.502	2.963	3.478	2.927	3.949
HCM Lane V/C Ratio	0.139	0.011	0.515	0.069	0.481	0.059
HCM Control Delay	9.5	8.6	13.4	8.9	12.6	9.3
HCM Lane LOS	A	A	B	A	B	A
HCM 95th-tile Q	0.5	0	2.9	0.2	2.6	0.2

Intersection

Intersection Delay, s/veh 18.2

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗	
Traffic Vol, veh/h	26	323	38	66	259	32	58	70	96	22	71	19
Future Vol, veh/h	26	323	38	66	259	32	58	70	96	22	71	19
Peak Hour Factor	0.96	0.96	0.96	0.89	0.89	0.89	0.81	0.81	0.84	0.69	0.69	0.69
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	27	336	40	74	291	36	72	86	114	32	103	28
Number of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			2			2		
HCM Control Delay	23.3			18.2			13.7			12.9		
HCM LOS	C			C			B			B		

Lane	NBLn1	NBLn2	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	100%	0%	100%	0%	100%	0%	100%	0%
Vol Thru, %	0%	42%	0%	89%	0%	89%	0%	79%
Vol Right, %	0%	58%	0%	11%	0%	11%	0%	21%
Sign Control	Stop							
Traffic Vol by Lane	58	166	26	361	66	291	22	90
LT Vol	58	0	26	0	66	0	22	0
Through Vol	0	70	0	323	0	259	0	71
RT Vol	0	96	0	38	0	32	0	19
Lane Flow Rate	72	201	27	376	74	327	32	130
Geometry Grp	7	7	7	7	7	7	7	7
Degree of Util (X)	0.159	0.393	0.055	0.706	0.152	0.615	0.073	0.274
Departure Headway (Hd)	7.972	7.044	7.343	6.757	7.365	6.776	8.242	7.575
Convergence, Y/N	Yes							
Cap	450	510	487	534	486	533	434	473
Service Time	5.723	4.794	5.089	4.503	5.113	4.523	6.001	5.334
HCM Lane V/C Ratio	0.16	0.394	0.055	0.704	0.152	0.614	0.074	0.275
HCM Control Delay	12.2	14.3	10.5	24.2	11.4	19.8	11.7	13.2
HCM Lane LOS	B	B	B	C	B	C	B	B
HCM 95th-tile Q	0.6	1.9	0.2	5.6	0.5	4.1	0.2	1.1

Intersection

Intersection Delay, s/veh 9.3

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Future Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Peak Hour Factor	0.73	0.73	0.73	0.63	0.63	0.63	0.82	0.82	0.82	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	25	23	36	19	6	14	24	246	21	15	149	21
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	8.5			8.3			9.9			8.9		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	8%	30%	48%	8%
Vol Thru, %	85%	28%	16%	81%
Vol Right, %	7%	43%	36%	11%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	239	61	25	170
LT Vol	20	18	12	14
Through Vol	202	17	4	137
RT Vol	17	26	9	19
Lane Flow Rate	291	84	40	185
Geometry Grp	1	1	1	1
Degree of Util (X)	0.357	0.113	0.055	0.231
Departure Headway (Hd)	4.415	4.854	4.994	4.502
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	813	737	715	796
Service Time	2.444	2.895	3.041	2.534
HCM Lane V/C Ratio	0.358	0.114	0.056	0.232
HCM Control Delay	9.9	8.5	8.3	8.9
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	1.6	0.4	0.2	0.9

Intersection

Intersection Delay, s/veh 16.2

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓			↔			↔	
Traffic Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24
Future Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.92	0.92	0.92	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	20	461	16	16	361	26	14	11	35	13	19	32
Number of Lanes	1	1	0	1	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			2			2		
HCM Control Delay	18.8			15			9.7			9.8		
HCM LOS	C			B			A			A		

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1
Vol Left, %	24%	100%	0%	100%	0%	21%
Vol Thru, %	18%	0%	97%	0%	93%	29%
Vol Right, %	58%	0%	3%	0%	7%	50%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	55	17	415	15	360	48
LT Vol	13	17	0	15	0	10
Through Vol	10	0	401	0	336	14
RT Vol	32	0	14	0	24	24
Lane Flow Rate	60	20	477	16	387	64
Geometry Grp	2	7	7	7	7	2
Degree of Util (X)	0.1	0.031	0.693	0.027	0.58	0.108
Departure Headway (Hd)	6.02	5.863	5.335	5.945	5.393	6.051
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes
Cap	597	614	682	606	674	594
Service Time	4.043	3.563	3.035	3.645	3.093	4.073
HCM Lane V/C Ratio	0.101	0.033	0.699	0.026	0.574	0.108
HCM Control Delay	9.7	8.8	19.2	8.8	15.3	9.8
HCM Lane LOS	A	A	C	A	C	A
HCM 95th-tile Q	0.3	0.1	5.6	0.1	3.7	0.4

APPENDIX G

SYNCHRO RESULTS MIXED STOP-CONTROLLED

Intersection

Int Delay, s/veh 2.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗		↖ ↗	↖ ↗	
Traffic Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Future Vol, veh/h	3	244	25	11	244	8	23	7	16	5	16	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	50	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	75	75	84	84	84	68	68	68	50	50	50
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	325	33	13	290	10	34	10	24	10	32	6

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	300	0	0	358	0	0	690	676	342	688	687	295
Stage 1	-	-	-	-	-	-	350	350	-	321	321	-
Stage 2	-	-	-	-	-	-	340	326	-	367	366	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1261	-	-	1201	-	-	359	375	701	360	370	744
Stage 1	-	-	-	-	-	-	666	633	-	691	652	-
Stage 2	-	-	-	-	-	-	675	648	-	653	623	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1261	-	-	1201	-	-	329	370	701	337	365	744
Mov Cap-2 Maneuver	-	-	-	-	-	-	329	370	-	337	365	-
Stage 1	-	-	-	-	-	-	664	631	-	689	645	-
Stage 2	-	-	-	-	-	-	629	641	-	619	621	-

Approach	EB	WB	NB	SB				
HCM Control Delay, s	0.1	0.3	15.4	15.7				
HCM LOS			C	C				
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	412	1261	-	-	1201	-	-	383
HCM Lane V/C Ratio	0.164	0.003	-	-	0.011	-	-	0.125
HCM Control Delay (s)	15.4	7.9	-	-	8	-	-	15.7
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.6	0	-	-	0	-	-	0.4

Intersection

Intersection Delay, s/veh 16.1

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓		↑	↓		↑	↓	
Traffic Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Future Vol, veh/h	14	185	66	97	214	18	32	58	66	16	117	21
Peak Hour Factor	0.77	0.77	0.77	0.86	0.86	0.86	0.59	0.59	0.59	0.70	0.70	0.70
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	18	240	86	113	249	21	54	98	112	23	167	30
Number of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			2			2		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			2			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	2			2			2			2		
HCM Control Delay	19.4			15.5			13.9			14.6		
HCM LOS	C			C			B			B		

Lane	NBLn1	NBLn2	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	100%	0%	100%	0%	100%	0%	100%	0%
Vol Thru, %	0%	47%	0%	74%	0%	92%	0%	85%
Vol Right, %	0%	53%	0%	26%	0%	8%	0%	15%
Sign Control	Stop							
Traffic Vol by Lane	32	124	14	251	97	232	16	138
LT Vol	32	0	14	0	97	0	16	0
Through Vol	0	58	0	185	0	214	0	117
RT Vol	0	66	0	66	0	18	0	21
Lane Flow Rate	54	210	18	326	113	270	23	197
Geometry Grp	7	7	7	7	7	7	7	7
Degree of Util (X)	0.119	0.408	0.038	0.615	0.233	0.515	0.051	0.402
Departure Headway (Hd)	7.888	6.993	7.491	6.792	7.443	6.876	7.966	7.343
Convergence, Y/N	Yes							
Cap	454	515	478	531	482	523	449	489
Service Time	5.643	4.747	5.241	4.541	5.194	4.627	5.722	5.099
HCM Lane V/C Ratio	0.119	0.408	0.038	0.614	0.234	0.516	0.051	0.403
HCM Control Delay	11.7	14.5	10.5	19.9	12.5	16.8	11.2	15
HCM Lane LOS	B	B	B	C	B	C	B	B
HCM 95th-tile Q	0.4	2	0.1	4.1	0.9	2.9	0.2	1.9

Intersection

Int Delay, s/veh

2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Future Vol, veh/h	5	239	6	17	325	14	6	8	13	9	14	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	100	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	94	94	94	61	61	61	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	6	295	7	18	346	15	10	13	21	12	19	21

Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	361	0	0	302	0	0	721	708	299	718	704	354
Stage 1	-	-	-	-	-	-	311	311	-	390	390	-
Stage 2	-	-	-	-	-	-	410	397	-	328	314	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1198	-	-	1259	-	-	343	360	741	344	361	690
Stage 1	-	-	-	-	-	-	699	658	-	634	608	-
Stage 2	-	-	-	-	-	-	619	603	-	685	656	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1198	-	-	1259	-	-	315	353	741	320	354	690
Mov Cap-2 Maneuver	-	-	-	-	-	-	315	353	-	320	354	-
Stage 1	-	-	-	-	-	-	696	655	-	631	599	-
Stage 2	-	-	-	-	-	-	573	595	-	649	653	-

Approach	EB	WB	NB	SB							
HCM Control Delay, s	0.2	0.4	13.7	14.5							
HCM LOS		B	B								
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Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	456	1198	-	-	1259	-	-	429			
HCM Lane V/C Ratio	0.097	0.005	-	-	0.014	-	-	0.121			
HCM Control Delay (s)	13.7	8	-	-	7.9	-	-	14.5			
HCM Lane LOS	B	A	-	-	A	-	-	B			
HCM 95th %tile Q(veh)	0.3	0	-	-	0	-	-	0.4			

Intersection

Int Delay, s/veh 2.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Future Vol, veh/h	2	7	33	13	4	7	20	159	14	5	264	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	58	58	75	75	75	57	57	57	66	66	66
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	12	57	17	5	9	35	279	25	8	400	20

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	795	800	410	823	798	292	420	0	0	304	0	0
Stage 1	426	426	-	362	362	-	-	-	-	-	-	-
Stage 2	369	374	-	461	436	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	305	318	642	292	319	747	1139	-	-	1257	-	-
Stage 1	606	586	-	657	625	-	-	-	-	-	-	-
Stage 2	651	618	-	581	580	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	287	304	642	249	305	747	1139	-	-	1257	-	-
Mov Cap-2 Maneuver	287	304	-	249	305	-	-	-	-	-	-	-
Stage 1	584	581	-	633	602	-	-	-	-	-	-	-
Stage 2	614	595	-	514	575	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	13.1	17.5			0.9			0.1				
HCM LOS	B	C										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1139	-	-	516	321	1257	-	-				
HCM Lane V/C Ratio	0.031	-	-	0.14	0.1	0.006	-	-				
HCM Control Delay (s)	8.3	0	-	13.1	17.5	7.9	0	-				
HCM Lane LOS	A	A	-	B	C	A	A	-				
HCM 95th %tile Q(veh)	0.1	-	-	0.5	0.3	0	-	-				

Intersection

Int Delay, s/veh 2.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Future Vol, veh/h	7	324	17	38	277	20	29	9	33	8	14	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	50	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	89	89	89	80	80	80	69	69	69
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	338	18	43	311	22	36	11	41	12	20	4

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	333	0	0	356	0	0	781	780	347	795	778	322
Stage 1	-	-	-	-	-	-	361	361	-	408	408	-
Stage 2	-	-	-	-	-	-	420	419	-	387	370	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1226	-	-	1203	-	-	312	327	696	305	328	719
Stage 1	-	-	-	-	-	-	657	626	-	620	597	-
Stage 2	-	-	-	-	-	-	611	590	-	637	620	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1226	-	-	1203	-	-	285	313	696	270	314	719
Mov Cap-2 Maneuver	-	-	-	-	-	-	285	313	-	270	314	-
Stage 1	-	-	-	-	-	-	653	622	-	616	576	-
Stage 2	-	-	-	-	-	-	565	569	-	585	616	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.2	0.9			16.6			17.7			
HCM LOS					C			C			
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Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBLn1		
Capacity (veh/h)	399	1226	-	-	1203	-	-	-	319		
HCM Lane V/C Ratio	0.222	0.006	-	-	0.035	-	-	-	0.114		
HCM Control Delay (s)	16.6	8	-	-	8.1	-	-	-	17.7		
HCM Lane LOS	C	A	-	-	A	-	-	-	C		
HCM 95th %tile Q(veh)	0.8	0	-	-	0.1	-	-	-	0.4		

Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24
Future Vol, veh/h	17	401	14	15	336	24	13	10	32	10	14	24
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	100	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	93	93	93	92	92	92	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	20	461	16	16	361	26	14	11	35	13	19	32

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	387	0	0	477	0	0	941	928	469	938	923	374
Stage 1	-	-	-	-	-	-	509	509	-	406	406	-
Stage 2	-	-	-	-	-	-	432	419	-	532	517	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1171	-	-	1085	-	-	243	268	594	244	270	672
Stage 1	-	-	-	-	-	-	547	538	-	622	598	-
Stage 2	-	-	-	-	-	-	602	590	-	531	534	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1171	-	-	1085	-	-	214	259	594	217	261	672
Mov Cap-2 Maneuver	-	-	-	-	-	-	214	259	-	217	261	-
Stage 1	-	-	-	-	-	-	538	529	-	611	589	-
Stage 2	-	-	-	-	-	-	547	581	-	481	525	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	0.3		17		17.4		
HCM LOS				C		C		
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Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	359	1171	-	-	1085	-	-	354
HCM Lane V/C Ratio	0.167	0.017	-	-	0.015	-	-	0.181
HCM Control Delay (s)	17	8.1	-	-	8.4	-	-	17.4
HCM Lane LOS	C	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.6	0.1	-	-	0	-	-	0.7

Intersection

Int Delay, s/veh 3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Future Vol, veh/h	18	17	26	12	4	9	20	202	17	14	137	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	63	63	63	82	82	82	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	25	23	36	19	6	14	24	246	21	15	149	21

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	505	505	160	524	505	257	170	0	0	267	0	0
Stage 1	190	190	-	305	305	-	-	-	-	-	-	-
Stage 2	315	315	-	219	200	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	478	470	885	464	470	782	1407	-	-	1297	-	-
Stage 1	812	743	-	705	662	-	-	-	-	-	-	-
Stage 2	696	656	-	783	736	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	453	454	885	417	454	782	1407	-	-	1297	-	-
Mov Cap-2 Maneuver	453	454	-	417	454	-	-	-	-	-	-	-
Stage 1	796	733	-	691	649	-	-	-	-	-	-	-
Stage 2	663	643	-	718	726	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	12.4	12.7	0.6	0.6
HCM LOS	B	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1407	-	-	572	509	1297	-	-
HCM Lane V/C Ratio	0.017	-	-	0.146	0.078	0.012	-	-
HCM Control Delay (s)	7.6	0	-	12.4	12.7	7.8	0	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.5	0.3	0	-	-